

Woodford County High School Emergency Structural Repairs

Versailles, Kentucky

for the

Woodford County Board of Education

330 Pisgah Pike , Kentucky 40383
p 859.879.4600

RTA # 2133



101 old lafayette avenue
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enhancing education through great design

STRUCTURAL ENGINEER: STRUCTURAL DESIGN GROUP, INC.
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PROJECT SITE ADDRESS:

180 Frankfort Street
Versailles, Kentucky
40383

VICINITY MAP

A map of the state of Kentucky with county boundaries. A black dot in the central part of the state indicates the location of Versailles, Kentucky. Neighboring states are labeled: Indiana, Ohio, West Virginia, Virginia, Tennessee, and Missouri. A north arrow is in the bottom right corner.

PROJECT VICINITY MAP

A detailed map of the Versailles, Kentucky area, showing streets and landmarks. A specific location is marked with a black dot and labeled 'SITE'. A north arrow is in the bottom right corner.

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COVER SHEET
WOODFORD COUNTY HIGH SCHOOL EMERGENCY STRUCTURAL REPAIRS
FOR:
WOODFORD COUNTY BOARD OF EDUCATION
330 Pisgah Pike , Kentucky 40383

BG# 22-158
Project No: 2133
Drawn By: TS
Rev'd By: MH
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CONSTRUCTION DOCUMENTS
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DATE ISSUED:
11/05/2021

STRUCTURAL NOTES

THE STRUCTURAL NOTES DEFINE GENERAL DESIGN AND MATERIAL REQUIREMENTS AND ARE INTENDED TO SUPPLEMENT, BUT NOT REPLACE, THE PROJECT SPECIFICATIONS

DESIGN CRITERIA

- Building Code: 2018 Kentucky Building Code and ASCE 7-10 (except Chapter 14 and Appendix 11A)
 - Building Risk Category: III
- Design Loads
 - Uniform Floor Live Loads (reduced per Building Code, UNO)

Gymnasiums: Main/Balconies 100 psf
 - Concentrated Floor Live Loads (distributed over 2.5 ft x 2.5 ft, UNO)

Schools 1,000 lbs

GENERAL

- Reference to standards or specifications of technical societies, organizations, or associations means the standard or specification referenced by the governing Building Code shown on the Drawings, unless specifically noted otherwise.
- Material, workmanship, and design shall conform to the referenced Building Code.
- For dimensions not shown in the Structural Drawings, see the Architectural Drawings.
- Contractor responsibilities include, but are not limited to, the following:
 - The structure is stable only in its completed form. Temporary supports required for stability during all intermediate stages of construction shall be designed, furnished, and installed by the Contractor.
 - Contractor has sole responsibility for jobsite safety and complying with all health and safety precautions as required by any regulatory agency. In performing construction observation visits to the jobsite, the Structural Engineer will have no control over, nor responsibility for, the Contractor's means, methods, sequences, techniques, or Procedures in performing the work.
 - Contractor is responsible for locating concrete reinforcement prior to installation of post-installed anchors, through bolts, or other post-installed items in concrete. Existing reinforcement including post-tensioning tendons shall not be cut or otherwise damaged while installing post-installed anchors.
 - Contractor shall visit the project site prior to placing a bid to perform any structural repair work in order to observe the existing conditions of the structure.
 - Contractor shall coordinate all structural repair work with all trades and existing conditions and notify the structural engineer of any conflicts before starting related work. Related work can start once an approved solution has been issued.
- Existing and Unforeseen Conditions
 - Contractor shall field verify all existing conditions, elevations, and site conditions prior to construction and fabrication. Contractor shall immediately notify Structural Engineer of any existing conditions that are in conflict with the Structural Documents.
 - Shop drawing submittals shall be based on field verified dimensions and conditions only. Contractor shall clearly show actual field dimensions on shop drawings.
 - Existing dimensions, elevations, and other information shown in the Structural Drawings are based on the following Documents:

WOODFORD COUNTY HIGH SCHOOL, prepared by Luckett & Farley Inc. Architects, dated Sept 9, 1963

SUBMITTALS

- Shop Drawings and Submittals
 - Reproduction of Structural Drawings for shop drawings is not permitted.
 - Electronic drawing files will not be provided to the Contractor.
 - Review of shop drawings will be for conformance with the Construction Documents regarding arrangement and sizes of members and the Contractor's interpretation of the design loads, if applicable, and construction document details. Such review shall not relieve the Contractor of the full responsibility to comply with the construction documents.
- Submittals
 - The Structural Quality Assurance Plan and Specifications identify the required submittals. Prior to (or with) the first submittal, Contractor shall submit a list of all required submittals for Engineer's review.

FOUNDATION

- No Geotechnical report is available.
- Soil Bearing Capacity: Isolated Footings 2500 psf
(assumed, Special Inspector to verify)

REINFORCEMENT

- Reinforcing Bars: ASTM A615, Grade 60
 - Reinforcing bars are not to be welded.
- Reinforcement Placement (UNO)
 - Concrete Reinforcement Clear Cover Below Grade: Unformed 3-in
 Formed 2-in
- Reinforcement Splices
 - Reinforcement marked "Continuous" can be spliced at locations determined by Contractor. All other reinforcement shall be spliced only at locations shown or noted, unless approved in writing by Structural Engineer.
 - Splice Lengths (UNO)

Concrete Reinforcement: Class B Tension Lap

CAST-IN-PLACE CONCRETE

- Concrete Properties
 - Normal Weight Structural Concrete

	28-Day, f'c (min)	w/cm Ratio (max.)	Entrained Air
Footings (Isolated/Continuous)	3,000 psi	----	None Required
Infill Slabs	4,000 psi	0.48	None Required
All Other Concrete	5,000 psi	0.40	5.0 +/- 1.5%

Note: All concrete shall be assigned the exposure classes F0, S0, W0, and C0; except concrete in Aggressive Environment shall be assigned the exposure classes F3, S3, W1, and C2 (see ACI 318).
- Construction Joint Locations: No horizontal construction joints are permitted except as shown on the Structural Drawings. Obtain written consent for additional joints.

NON-SHRINK GROUTING

- Non-shrink grout under steel base plates shall be non-metallic with minimum compressive strength of 5000 psi at 28 days.
- Non-shrink grout used for patching, repair, and other specific applications shall be submitted for review and approval by engineer.

STRUCTURAL STEEL

- Steel Shapes
 - W-Shapes: ASTM A992 (Grade 50)
 - Angles, Channels, Plates, UNO: ASTM A36
 - Square/Rectangular/Round Hollow Structural Sections (HSS): ASTM A500, Grade B
- Anchor Rods, Bolts, and Studs
 - Anchor Rods: ASTM F1554, Grade 36. Headed Rods or threaded rods with plate washer and heavy hex nut.
 - Bolts: 3/4-in Diameter A325 minimum. All connections may be bearing type, UNO. Design bearing type connections for load values with threads included in the shear plane. Submit proposed bolt tightening procedure for review.
- Structural steel shall be fabricated and erected according to the "Specification for Structural Steel Buildings" referenced in the applicable Building Code.
- Shop Drawings: Submittal shall adequately depict structural members and connections.
- Welders shall be qualified for the work performed in accordance with AWS D1.1. welder qualifications shall be certified by the local building authority and verified by the Contractor and the Special Inspector.
- Written welding procedures for shop and field welding of all structural steel shall be submitted to the Structural Engineer and the Special Inspector for review and approval. Do not fabricate steel until the welding procedures have been approved. The approved written welding procedures shall be strictly adhered to during the fabrication and field erection of all structural steel.

POST-INSTALLED ANCHORS

- Post-installed anchors shall only be installed where indicated on the structural drawings, unless approved by engineer of record.
- The below products are the design basis for this project. Product diameter and embedment shall be as shown in the details. Install products IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). Refer to the project building code and/or evaluation report for special inspections and proof load requirements. Substitution requests for products other than those listed below may be submitted by the contractor to the Engineer-of-Record (EOR) for review. Substitutions will only be considered for products having a research report recognizing the product for the appropriate application under the project building code. Substitution requests shall include calculations that demonstrate the substituted product is capable of achieving the equivalent performance values of the design basis product.
- For Anchoring into Concrete
 - Expansion Anchors: Hilti Kwik Bolt TZ (ICC-ES ESR-1917), Simpson Strong-Bolt 2 (ICC-ES ESR-3037), Dewart/Powers Power-Stud+ SD1 (ICC-ES ESR-2818), or Dewart/Powers Power-Stud+ SD2 (ICC-ES ESR-2502).
Minimum embedment = 6 times anchor diameter, UNO.
 - Screw Anchors: Simpson Titen-HD (Concrete: ICC-ES ESR-2713; Grouted Masonry: ICC-ES ESR-1056) or Dewart Screw Bolt+ (ICC-ES ESR-3889), Hilti Kwik HUS-EZ (ICC-ES ESR-3027).
Minimum Embedment = 6 times anchor diameter, UNO.
 - Adhesive Anchors
 - Adhesive anchors shall be installed in concrete having a minimum age of 21 days at time of anchor installation.
 - Adhesive anchors identified in the drawings as installed in a horizontal or upwardly inclined orientation to resist sustained tensile loads shall be installed by certified installers.
 - All-thread steel rods conforming ASTM A36 or bolts conforming to ASTM A307, Grade A or, both zinc plated in accordance with ASTM B633 or reinforcing bars conforming to ASTM A615, Grade 60.
 - All-thread steel rods conforming ASTM A36 or bolts conforming to ASTM A307, Grade A or, both zinc plated in accordance with ASTM B633 or reinforcing bars conforming to ASTM A615, Grade 60.
 - Adhesive for rebar and anchors shall have been tested in accordance with ACI 355.4 and ICC-ES AC308 for cracked concrete and seismic applications. Design bond strength has been based on CRACKED CONCRETE, ACI 355.4 temperature category B, and installations into dry holes drilled using a hammer drill into concrete that has cured for at least 21 days. Adhesive anchors shall be installed by a certified adhesive anchor installer PER ACI 318 17.8.2.2 where INDICATED on the contract documents. Installations requiring certified installers shall be inspected per ACI 318 17.8.2.4.
 - Adhesive conforming to Simpson AT-XP (IAPMO-UES ER-263), Simpson SET-XP (ICC-ES ESR-2508), Dewart/Powers Pure110+ (ICC-ES ESR-3298), Dewart/Powers Dewart AC200+ Adhesive (ICC-ES ESR-4027), Hilti HIT-HY 200 Safe Set Fast Cure Adhesive (ICC-ES ESR-3187), Hilti HIT-RE 500 V3 SAFE Set Adhesive (ICC-ES ESR-3814).
Minimum Embedment = 12 times anchor diameter, UNO.
- Contractor shall arrange for an anchor manufacturer's representative to provide onsite installation training for all of their anchoring products specified. The structural Engineer of record must receive documented confirmation that all of the contractor's personnel who install anchors are trained prior to the commencement of anchor installation.

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STRUCTURAL QUALITY ASSURANCE PLAN

GENERAL

This Structural Quality Assurance Plan includes:

- The Statement of Special Inspections which defines the scope of testing and inspection that is required for this project.
- The responsibilities of the Contractor.

Refer to other portions of the Construction Documents for Special Inspections required of architectural, mechanical, electrical, or other building components.

Special Inspector shall be hired by the Contractor and shall be approved by the Building Official and the Architect. Contractor shall submit with his bid the name and qualifications of the Structural Inspector(s).

Special Inspector will be hired by the Owner.

Special Inspector shall maintain records of inspections in accordance with Chapter 17 of the Building Code and shall distribute these records to the Building Official, Architect, and Structural Engineer on a weekly basis, unless noted otherwise below. Reports shall indicate that work inspected/tested was done in conformance to the Construction Documents. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected, they shall be brought to the attention of the Building Official, Architect, and Structural Engineer prior to completion of that phase of the work.

At the conclusion of the project, the Special Inspector shall submit a final report documenting required special inspections and correction of any discrepancies noted in the inspections.

STATEMENT OF SPECIAL INSPECTIONS

- The following tables contain material, components and work that require special inspection or testing:
 - Inspection Frequency, C – Continuous special inspection. Special inspection by the special inspector who is present when and where the work to be inspected is being performed.
 - Inspection Frequency, P – Periodic special inspection. Special inspection by the special inspector who is intermittently present where the work to be inspected has been or is being performed. For structural steel, observe the items on a random basis.
 - See Steel section for additional information for inspection tasks.

SOILS	INSPECTION FREQUENCY	REFERENCED STANDARD
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	P	
2. Verify excavations are extended to proper depth and have reached proper material.	P	Inspection is required after excavation is complete and prior to placement of structural fill.

CONCRETE CONSTRUCTION	INSPECTION FREQUENCY	REFERENCED STANDARD
1. Inspection of reinforcing steel placement and installation. Grade, size, quantity, quality, location, spacing, clearances.	P	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3 / IBC 1908.4
2. Inspection of anchors cast in concrete. Verify compliance of the following: diameter, grade, type, length, number, placement, and embedment depth.	C	ACI 318 17.8.2 / AISC 360 N5.7
3. Inspection of post-installed mechanical anchors installed in hardened concrete members: verify anchor type, anchor dimensions, hole diameter and cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque.	C	ACI 318 17.8.2 Use of post installed anchors must be approved by Structural Engineer
4. Inspection of post-installed adhesive anchors and reinforcing steel installed in hardened concrete members. Verify adhesive type, anchor rod dimensions, hole diameter and cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque.	C	ACI 318 17.8.2.4 Use of post installed anchors must be approved by Structural Engineer
5. Verify use of required design mix.	P	ACI 318 Ch. 19, 26.4.3 26.4.4 / IBC 1904.1, 1904.2, 1908.2, 1908.3
6. Sampling fresh concrete from concrete discharge. Mold one set of specimens for compressive strength testing for each 150 cubic yards or each 5,000 square feet of slab or wall surface area for each mix design placed in any one day. No fewer than five tests for a given class of concrete for the entire project. - Mold (5) 4x8-inch compressive strength cylinders, break and report (1) at 7-days, (3) at 28-days, or mold (4) 6x12-inch compressive strength cylinders, break and report (1) at 7-days, (2) at 28-days. - Remaining specimen(s) shall be broken as directed by the Structural Engineer. If compressive strengths do not appear adequate. - For each set molded, record: - Slump - Air Content - Unit Weight - Temperature, ambient and concrete - Batch and discharge times - Location and placement - Any pertinent information, such as addition of water, addition of admixtures, etc. - Report in writing on the same day as tests are performed. Reports of compressive strength tests shall contain the project identification name and number, date of concrete placement, name of concrete testing agency, concrete design compressive strength, location of concrete placement in structure, concrete mix proportions and materials, compressive breaking strength and type of break. - Verify compliance with construction documents.	C	ACI 318 26.5, 26.12 / IBC 1908.10 ASTM C172, ASTM C31
7. Inspection of concrete conveying and placement for proper application techniques.	C	ACI 318 26.5 / IBC 1908.6-1908.8
8. Inspection for maintenance of specified curing temperature and techniques.	P	ACI 318 26.5.3-26.5.5 / IBC 1908.9
9. Inspection of formwork for shape, location, and dimensions of the concrete member being formed.	P	ACI 318 26.11.2(b)

NON-SHRINK GROUTING	INSPECTION FREQUENCY	REFERENCED STANDARD
1. Compressive strength tests per ASTM C109. - Number of Tests; One test for each ten bags of grout used or minimum of one test of each day of grouting. - Cube Size: 2-inch x 2-inch - Test Schedule: (1) cube at 30days, (2) cubes at 7-days, (3) cubes at 28-days	C	
2. Perform one performance evaluation test prior placing grout under baseplates. Test shall be performed as outlined in ACI 301.1R-99	P	One test shall be performed at the beginning of the job prior to placement of grout under base plates.

STRUCTURAL STEEL	INSPECTION FREQUENCY	REFERENCED STANDARD
Where the following tasks have been performed by the fabricator's or erector's quality control program in accordance to Chapter N of AISC 360-10, it is permitted that these tasks be coordinated with the Special Inspector so that the inspection functions are performed by only one party. The Special Inspector shall review records of tasks performed by the erector's and fabricator's quality control program to verify completeness.	Obs. - Observe these items on a random basis. Operations need not be delayed pending these inspections.	
1. Inspection of steel framing to verify compliance with details shown on the approved construction documents including member locations, bracing, stiffening application of joint details at each connection, proper fasteners, etc.	Obs.	AISC 360-10 N5.7
2. Review the material test reports and certifications as listed below for compliance with the construction documents. - Main structural steel material test reports - Anchor rods and threaded rods test reports	Perf.	AISC 360-10 N5.2 & N3.2
3. Visual Inspection Tasks Prior to Welding - Welding procedure specifications (WPSs) available - Manufacturer certifications for welding consumables available - Material identification (type/grade) - Welder identification system. The fabricator or erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be the low-stress type.	Perf. Perf. Obs.	AISC 360-10 Table N5.4-1 AWS D1.1/D1.1M 6.3 AWS D1.1/D1.1M 6.2 AWS D1.1/D1.1M 6.4 (welder qualification) (identification system not required by AWS D1.1/D1.1M)
e. Fit-up of fillet welds - Dimensions (alignment, gaps at root) - Cleanliness (condition of steel surfaces) - Tacking (tack weld quality and location)	Obs.	AWS D1.1/D1.1M 5.22.1 AWS D1.1/D1.1M 5.15 AWS D1.1/D1.1M 5.18
f. Check welding equipment	Obs.	Only required for shop Fabrication.
4. Visual Inspection Tasks During Welding Use of qualified welders	Obs.	AISC 360-10 Table N5.4-2 AWS D1.1/D1.1M 6.4
b. Control and handling of welding consumables - Packaging - Exposure control	Obs.	AWS D1.1/D1.1M 5.3.1 AWS D1.1/D1.1M 5.3.2 (for SMAW), AWS D1.1/D1.1M 5.3.3 (for SAW)
c. No welding over cracked tack welds	Obs.	D1.1/D1.1M 5.18 (for SAW)
d. WPS followed	Obs.	AWS D1.1/D1.1M 6.3.3, 6.5.2, 5.5, 5.21
i. Setting on welding equipment		
ii. Travel speed		
iii. Selected welding materials		
iv. Shielding gas type/flow rate		
v. Preheat applied		
vi. Interpass temperature maintained (min./max.)		
vii. Proper position (F, V, H, OH)		
e. Welding techniques - Interpass and final cleaning - Each pass within profile limitations - Each pass meets quality requirements	Obs.	AWS D1.1/D1.1M 5.6, 5.7 AWS D1.1/D1.1M 6.5.2, 6.5.3, 5.24
5. Visual Inspection Tasks After Welding - Welds Cleaned - Size, length and location of welds - Welds meet visual acceptance criteria - Crack prohibition - Weld base-metal fusion - Crater cross section - Weld profiles - Weld size - Undercut - Porosity	Obs. Perf. Perf. Perf.	AISC 360-10 Table N5.4-3 AWS D1.1/D1.1M 5.30.1 AWS D1.1/D1.1M 6.5.1 AWS D1.1/D1.1M 6.5.3 AWS D1.1/D1.1M Table 6.1(1) AWS D1.1/D1.1M Table 6.1(2) AWS D1.1/D1.1M Table 6.1(3) AWS D1.1/D1.1M Table 6.1(4), 5.24 AWS D1.1/D1.1M Table 6.1(6) AWS D1.1/D1.1M Table 6.1(7) AWS D1.1/D1.1M Table 6.1(8)
d. Arc strikes	Perf.	AWS D1.1/D1.1M 5.29
e. Repair activities	Perf.	AWS D1.1/D1.1M 6.5.3, 5.26
f. Document acceptance or rejection of welded joint or member	Perf.	AWS D1.1/D1.1M 6.5.4, 6.5.5
g. No prohibited welds have been added without the approval of the EOR.	Obs.	
6. Nondestructive Testing (NDT) of Welded Joints		Ultrasonic testing (UT), magnetic particle testing (MT), penetrant testing (PT) and radiographic testing (RT), where required, shall be performed by Special Inspector in accordance with AWS D1.1/D1.1M. NDT of welds completed in a fabricator's shop may be performed by that fabricator when fabricator is AISC Certified or approved by the Building Official where applicable. When the fabricator performs the NDT, the Special inspection agency shall review the fabricator's NDT reports. All NDT of welds completed in the field shall be performed by the Special Inspector. Acceptance criteria shall be in accordance with AWS D1.1/D1.1M for statically loaded structures, unless otherwise designated in the design drawings or project specifications.
a. Document all NDT performed, identifying tested weld by location in the structure, piece mark and location. Concurrent to submitting NDT reports to EOR or owner submit to contractor.	Perf.	AISC 360-10 N5.5g
b. Review NDT test reports performed by fabricator	Perf.	AISC 360-10 N7
7. Inspection Tasks Prior to Bolting	Perform for 10% of all Snug tight joints if task is applicable and all pretension and slip critical joints. AISC 360-10 Table N5.6-1	
a. Manufacturer's certifications available for fastener materials	Perf.	RCSC 2.1 & 9.1
b. Fasteners marked in accordance with ASTM requirements	Perf.	RCSC Figure C-2.1 & 9.1 (Also See ASTM Standards)
c. Correct fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	Obs.	RCSC 2.3.2, 2.7.2 & 9.1
d. Correct bolting procedure selected for joint detail	Obs.	RCSC 4, & 8
e. Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	Obs.	RCSC 3, 9.4 & 9.3
f. Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used, not required for Snug tight bolts	Obs.	RCSC 7 & 9.2
g. Proper storage provided for bolts, nuts, washers and other fastener components	Obs.	RCSC 2.2.8 & 9.1
8. Inspection Tasks During Bolting	Perform for 10% of all Snug tight joints if task is applicable and all pretension and slip critical joints. Special Inspector need not be present during bolt pretensioning procedures. AISC 360-10 Table N5.6-2	
a. Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required	Obs.	RCSC 8.1 & 9.1
b. Joint brought to the snug-tight condition prior to the pretensioning operation	Obs.	RCSC 8.1 & 9.1
c. Fastener component not turned by the wrench prevented from rotating	Obs.	RCSC 8.2 & 9.2
d. Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edges	Obs.	RCSC 8.2 & 9.2
9. Inspection Tasks After Bolting		
a. Document acceptance or rejection of bolted connections	Perf.	AISC 360-10 Table N5.6-3

CONTRACTOR RESPONSIBILITIES

- Contractor shall pay for any additional structural testing/inspection required for work or materials not complying with the Construction Documents due to negligence or nonconformance and shall pay for any additional structural testing/inspection required for his convenience.
- Contractor is responsible to ensure that the Special Inspector is on site as required to perform all tasks required by Statement of Special Inspection. Any work that requires special inspection and is performed without the Special Inspector being present is subject to being demolished and reconstructed.
- Contractor has the following responsibilities to the Special Inspector:
 - Provide copy of Construction Documents to Special Inspector and latest addenda (include change orders and field orders prior to inspection of work contained therein).
 - Notify Special Inspector sufficiently in advance of operations to allow assignment of personnel and scheduling of tests.
 - Cooperate with Special Inspector and provide access to work.
 - Provide samples of materials to be tested in required quantities.
 - Provide storage space for Special Inspector's exclusive use, such as for storing and curing concrete testing samples.
 - Provide labor to assist Special Inspector in performing tests/inspections.
- Contractor shall perform the following:
 - CAST-IN-PLACE CONCRETE
 - Submit manufacturer's certification that reinforcing materials comply with Construction Documents.
 - Establish concrete mix design proportions in accordance with the specifications and ACI 318, Chapter 26.
 - Submit manufacturer's certification that concrete materials meet the requirements of the Construction Documents.
 - Submit manufacturer's data for tension and compression splitters.
 - NON-SHRINK GROUTING
 - Submit product data sheets for non-shrink grout that shows compliance with the Construction Documents and with ASTM C1107 for fluid or flowable grouts, prior to placement of grout.
 - STRUCTURAL STEEL
 - If fabricator or erector is not AISC certified, the fabricator and/or erector shall establish and maintain quality control procedures and perform inspections to ensure that their work is performed in accordance with the Section N of the Specification for Structural Steel Building, AISC 360-10 and the construction documents. Payment of these Quality control tests and inspections, except for all NDT of welds completed in the field by the Special Inspector, shall be by the fabricator and Erector.
 - Make available the documents listed in AISC 360-10 N3.2 in electronic or printed form for review by the EOR of the EOR's Designee prior to fabrication or erection unless otherwise required by the contract documents to be submitted.
 - If fabricator and erector are certified by the American Institute of Steel Construction (AISC) Quality Certification Program for Structural Steel Buildings submit certification.
 - At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the Building Official stating that the materials supplied and work performed by the fabricator are in accordance with the construction documents.
 - At completion of erection, the approved erector shall submit a certificate of compliance to the Building Official stating that the materials supplied and work performed by the erector are in accordance with the construction documents.
 - Provide non-destructive test (NDT) reports performed in shop by fabricator. Fabricator is responsible for cost of NDT performed in shop. Reports shall identify the tested weld by piece mark and location in the piece.
 - POST-INSTALLED ANCHORS
 - Contractor shall contact manufacturer's representative for product installation training. Submit a letter indicating that training has taken place.



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SOS Project No. 2021-17-05

STRUCTURAL QUALITY ASSURANCE PLAN
WOODFORD COUNTY HIGH SCHOOL EMERGENCY STRUCTURAL REPAIRS
FOR:
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330 Pisgah Pike, Kentucky 40383

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Project No: 2133
Drawn By: CCA
Rev'd By: CH/JDH

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CONSTRUCTION DOCUMENTS

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STRUCTURAL QUALITY
ASSURANCE PLAN
DATE ISSUED:
11/05/2021

LOCKER ROOM FOUNDATION PLANS
FOR:
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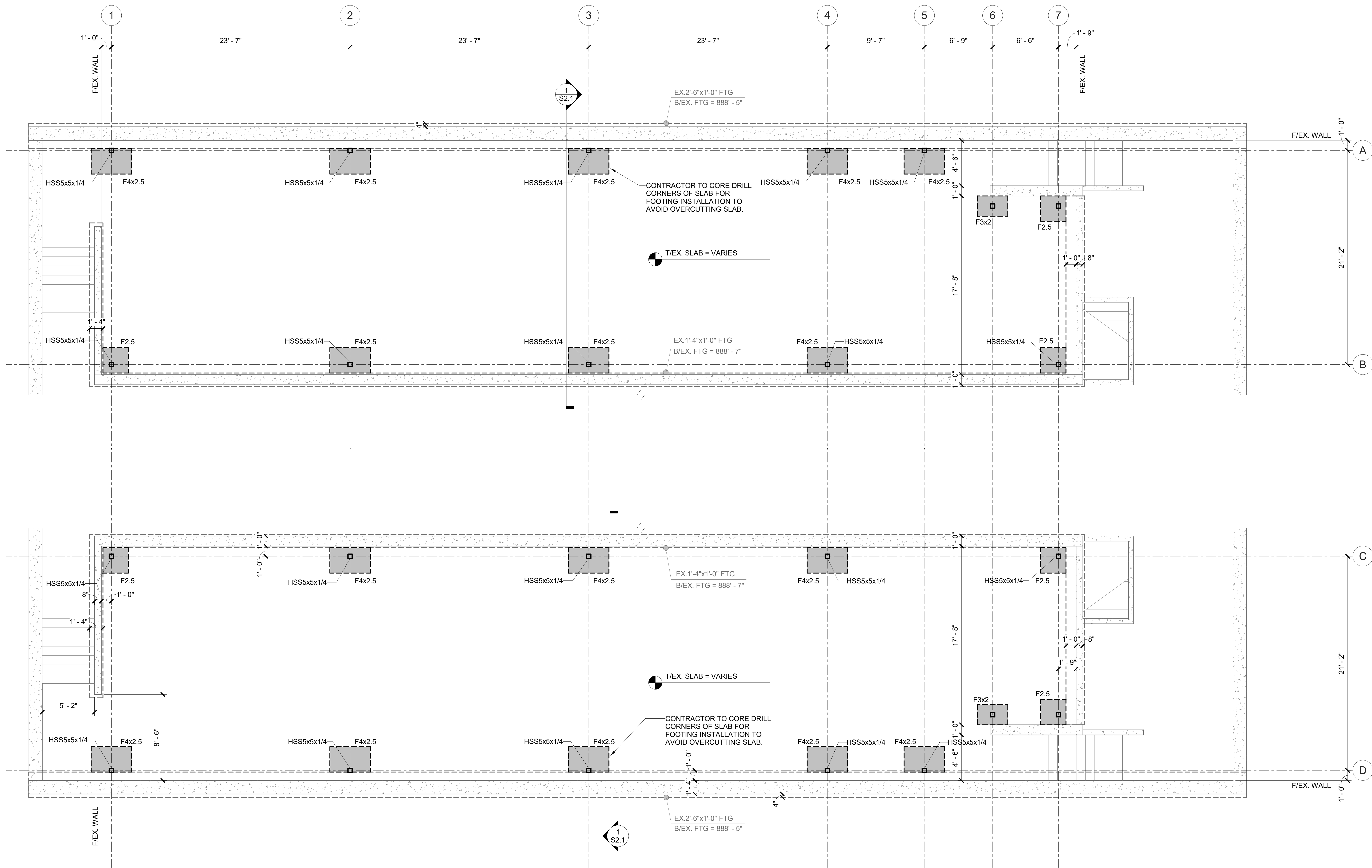
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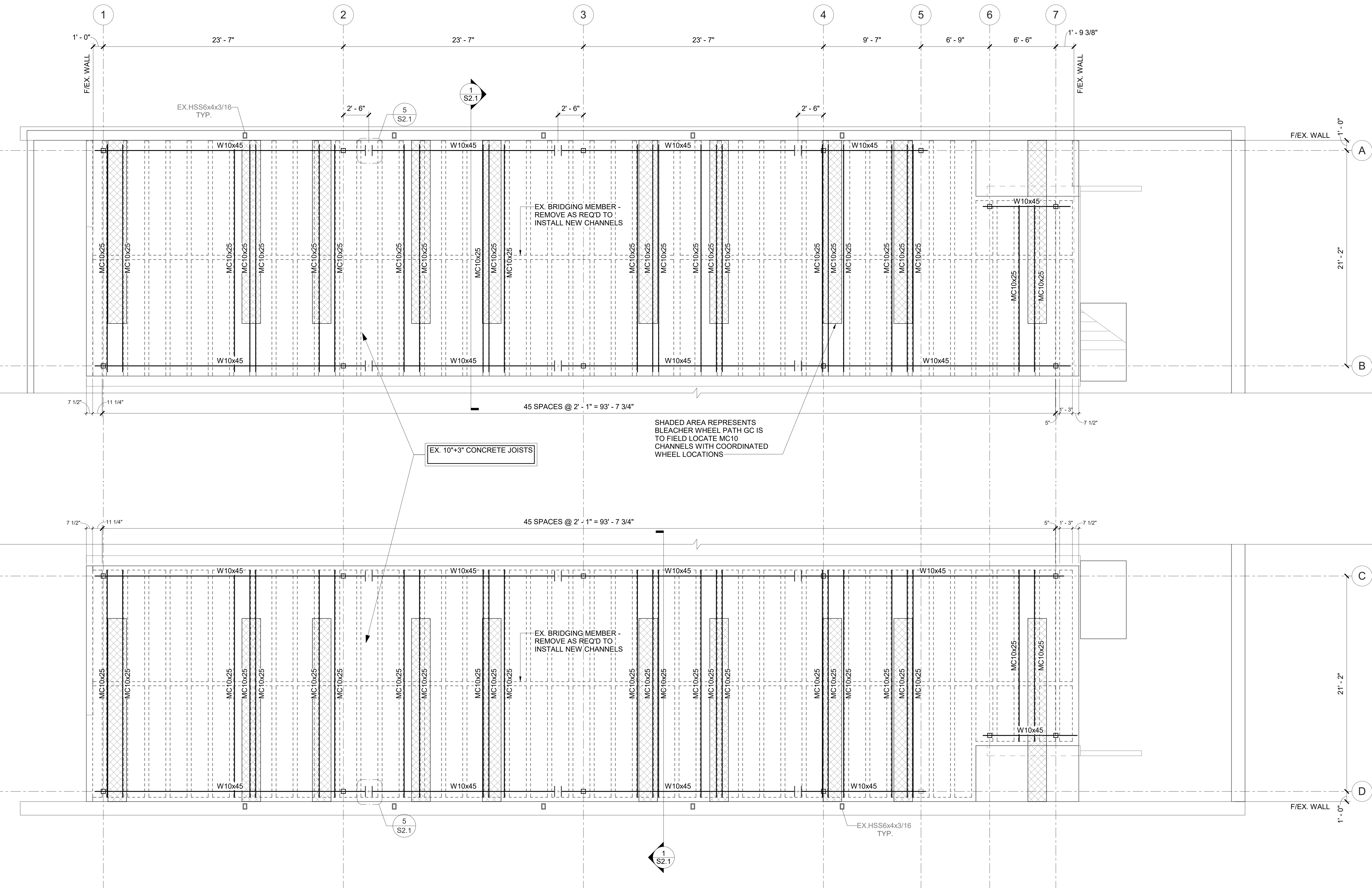
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S1.1
LOCKER ROOM FOUNDATION
PLANS
DATE ISSUED:
11/05/2021



LOCKER ROOM FOUNDATION PLANS

1/4" = 1'-0"



LOCKER ROOM FRAMING PLANS

1/4" = 1'-0"

LOCKER ROOM FRAMING PLANS
FOR:
WOODFORD COUNTY BOARD OF EDUCATION
330 Pitsgah Pike, Kentucky 40383

Structural Engineer
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BG# 22-158
Project No: 2133
Drawn By: CCA
Rev'd By: CH/JDH

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S1.2
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S1.2 Project No. 22-158

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