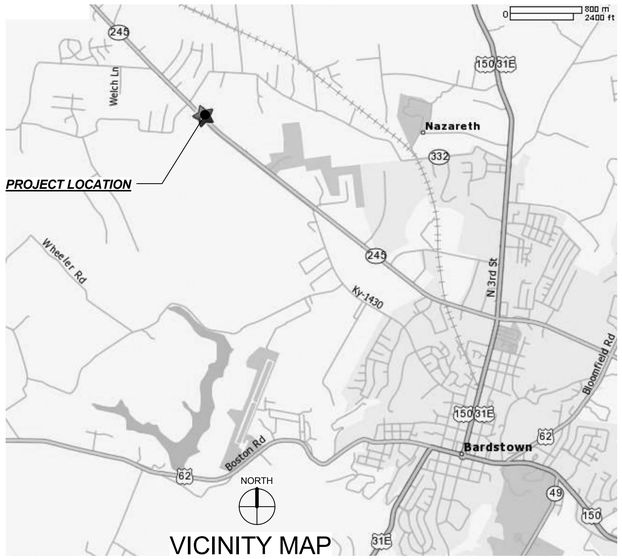
NELSON COUNTY SCHOOLS - WEST CAMPUS

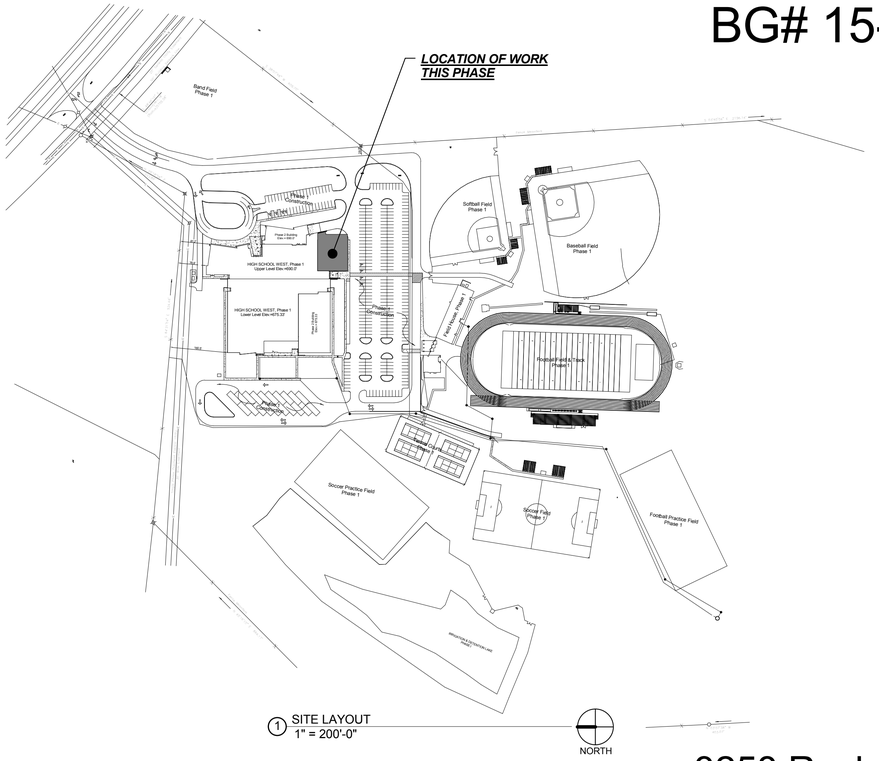
THOMAS NELSON HIGH SCHOOL - PHASE III (AUDITORIUM)

2885 NEW SHEPHERDSVILLE ROAD (HWY 245)
BARDSTOWN, KENTUCKY 40004

BG# 15-229



VICINITY MAP



1 SITE LAYOUT
1" = 200'-0"

NORTH

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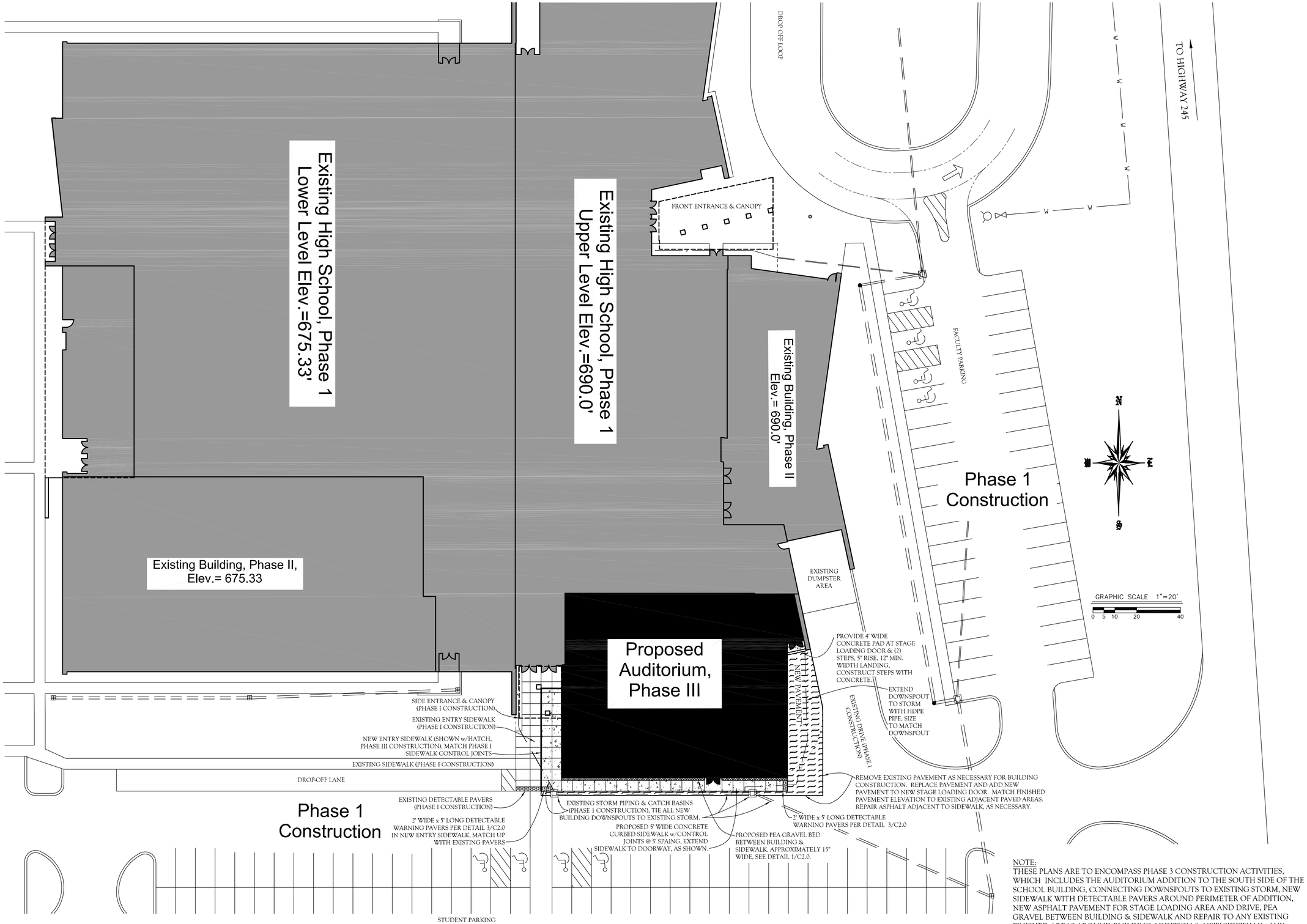
NELSON COUNTY BOARD OF EDUCATION

BARDSTOWN, KENTUCKY

studio kremer architects

3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101



NOTE:
THESE PLANS ARE TO ENCOMPASS PHASE 3 CONSTRUCTION ACTIVITIES, WHICH INCLUDES THE AUDITORIUM ADDITION TO THE SOUTH SIDE OF THE SCHOOL BUILDING, CONNECTING DOWNSPOUTS TO EXISTING STORM, NEW SIDEWALK WITH DETECTABLE PAVERS AROUND PERIMETER OF ADDITION, NEW ASPHALT PAVEMENT FOR STAGE LOADING AREA AND DRIVE, PEA GRAVEL BETWEEN BUILDING & SIDEWALK AND REPAIR TO ANY EXISTING FINISHED AREAS AROUND BUILDING ADDITION & NEW SIDEWALK. ANY LAWN AREAS THAT ARE DISTURBED DURING CONSTRUCTION NEED TO BE REPAIRED AND ENSURE THAT VEGETATION IS RE-ESTABLISHED.

CONSTRUCTION DOCUMENTS

studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101
studio

Horizon ENGINEERING, LLC
Civil Engineering & Land Surveying
111 North Second Street - P.O. Box 364
Bardstown, Ky. 40004
Phone: (502) 348-4330 - Fax: (502) 348-4340

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

SITE LAYOUT

NELSON COUNTY SCHOOLS
WEST CAMPUS
THOMAS NELSON H.S. PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245)
BARDSTOWN, KENTUCKY 40004

BG 15-229

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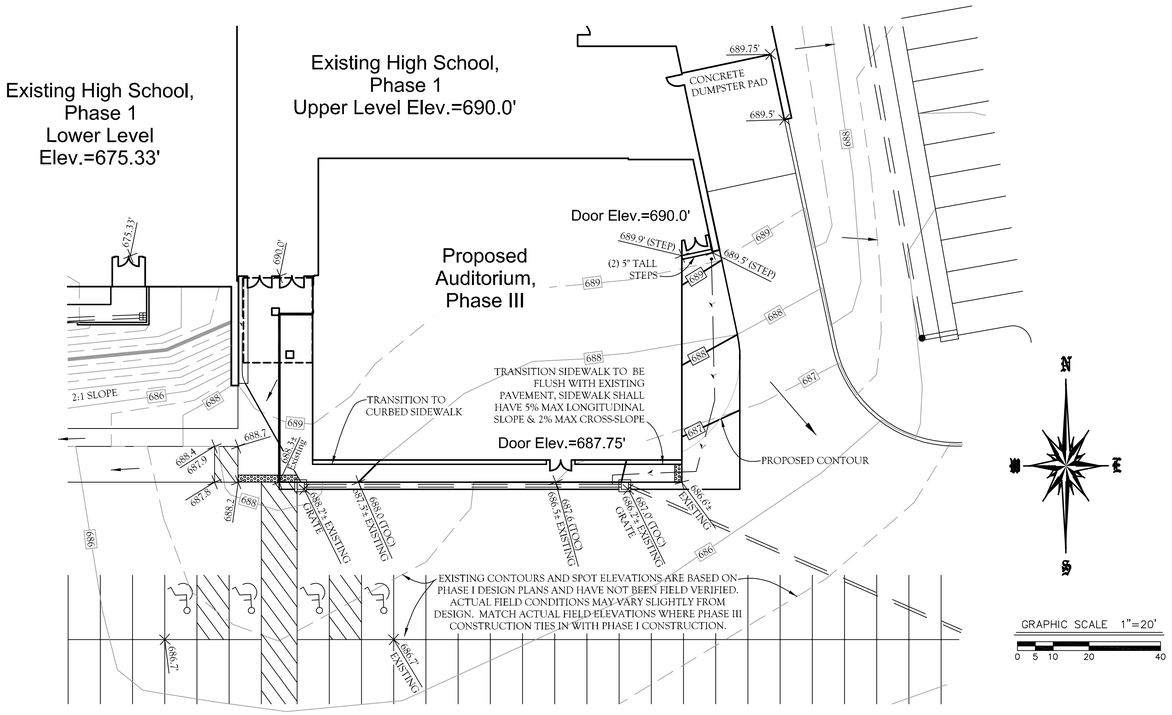
REVISIONS :

2011-02

PHASE - 3

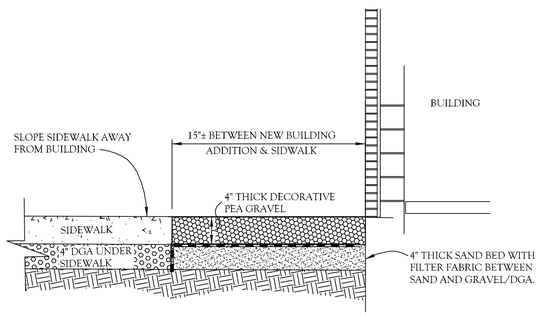
C1.0

HS



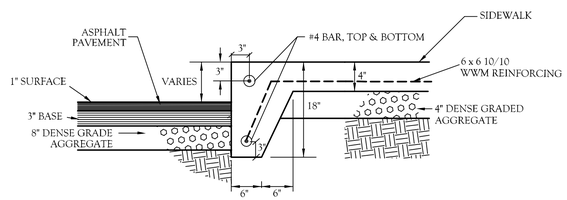
GRADING PLAN

NOTE: TOC = TOP OF CURB

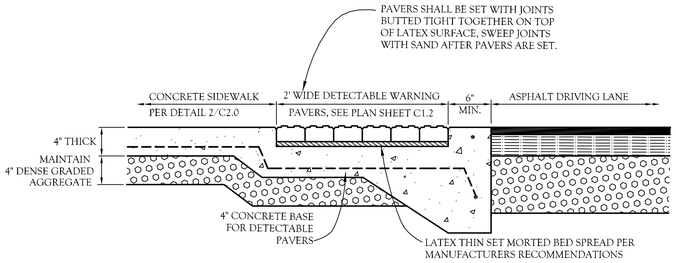


PEA GRAVEL BED — NOT TO SCALE — 1 / C2.0

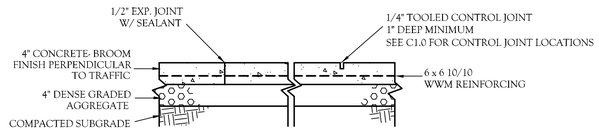
NOTES: 1. DO NOT DRAIN ANY DOWNSPOUTS INTO PEA GRAVEL.
2. DECORATIVE PEA GRAVEL SHALL BE ¾" DIAMETER, SMOOTH, ROUNDED STONE, LIGHT BROWN AND WHITE IN COLOR.



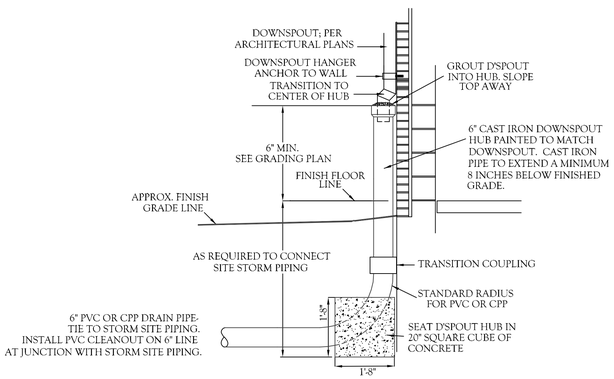
CONCRETE CURBED SIDEWALK & ASPHALT PAVING — NOT TO SCALE — 2 / C2.0



DETECTABLE WARNING PAVERS — NOT TO SCALE — 3 / C2.0



CONCRETE WALK — NOT TO SCALE — 4 / C2.0



NOTE: TYPICAL CONNECTION DETAIL AT ALL BUILDING DOWNSPOUTS. REFER TO ARCHITECTURAL PLANS FOR QUANTITY, LOCATION & SIZE OF DOWNSPOUTS.

DOWNSPOUT HUB — NOT TO SCALE — 5 / C2.0

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TEL 502.499.1100 FAX 499.1101
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111 North Second Street - P.O. Box 364
Bardstown, Ky. 40004
Phone: (502) 348-4330 - Fax: (502) 348-4340

NELSON COUNTY SCHOOL DISTRICT
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SITE GRADING & DETAILS

NELSON COUNTY SCHOOLS
WEST CAMPUS
THOMAS NELSON H.S. PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245)
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BG 15-229

DATE : 12/8/2015
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REVISIONS :

2011-02

PHASE - 3

C2.0

HS

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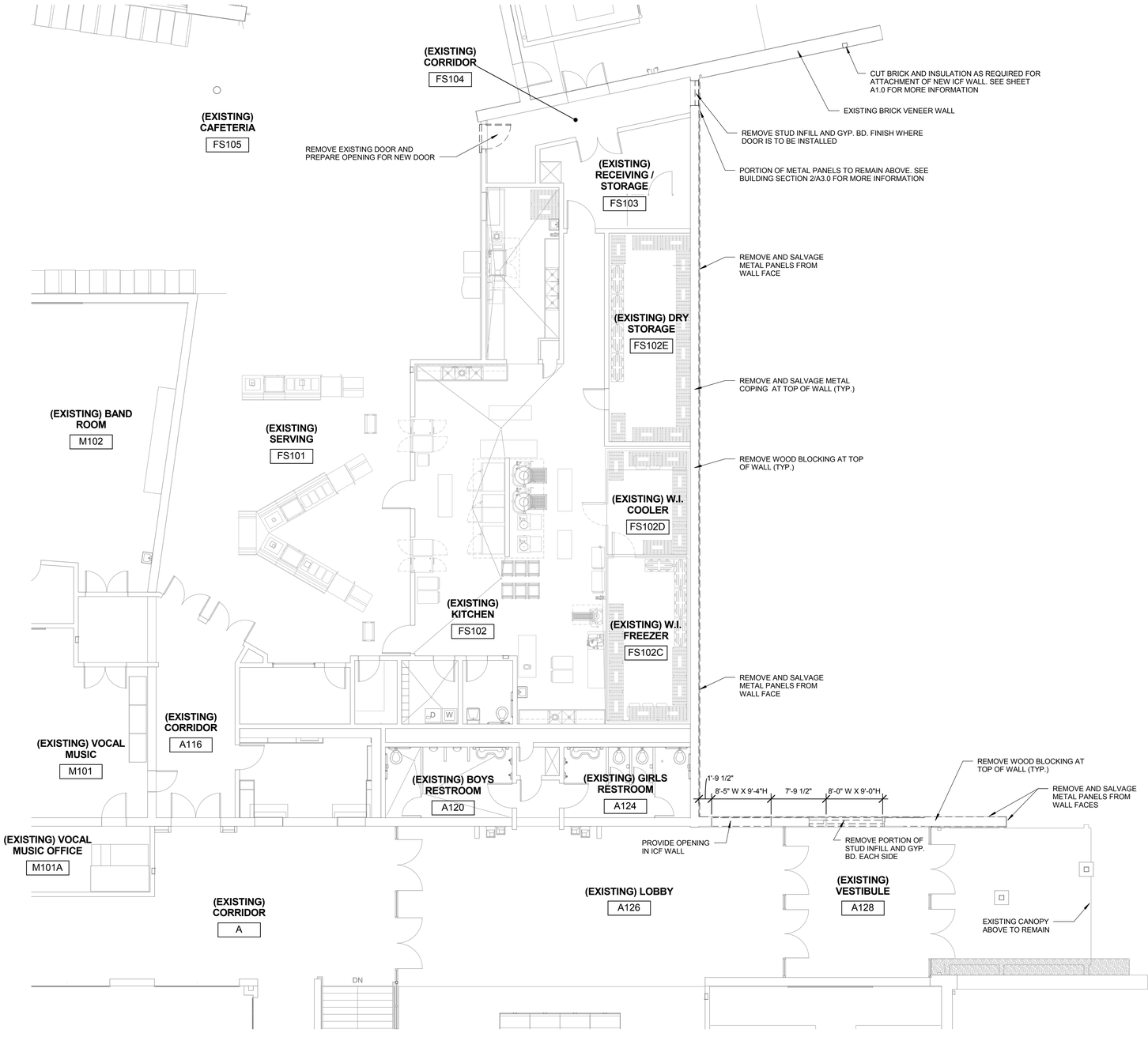
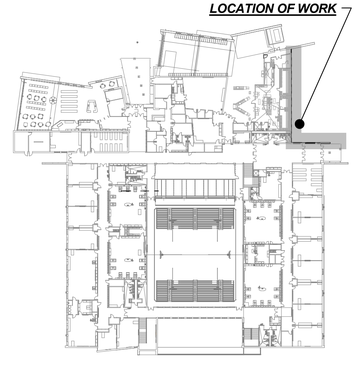
GENERAL DEMOLITION NOTES

1. CUTTING OF EXISTING ICF WALLS WHERE PORTIONS ARE TO BE REMOVED SHALL BE DONE FROM THE EXTERIOR OF THE WALL TO MINIMIZE DISTURBANCE TO THE INTERIOR.

2. ALL SALVAGED METAL WALL PANELS SHALL BE TURNED OVER TO OWNER.



1 DEMOLITION PLAN
1/8" = 1'-0"



2 KEY PLAN
1" = 80'-0"

CONSTRUCTION DOCUMENTS

studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

DEMOLITION PLAN
NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

DESIGNED TO EARN THE ENERGY STAR

BG 15-229

DATE : 12/8/2015
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REVISIONS :

2011-02

PHASE - 3

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STRUCTURAL GENERAL NOTES

STRUCTURAL DESIGN CRITERIA

1. APPLICABLE BUILDING CODES:
   A. 2013 KENTUCKY BUILDING CODE
   B. 2012 INTERNATIONAL BUILDING CODE
   C. ASCE STANDARD: ASCE 7–10

2. PROJECT LOCATION: BARDSTOWN, KENTUCKY (NELSON COUNTY)

3. DESIGN LOADS:

   A. FLOOR LIVE LOADS
      1. SLABS ON GRADE.....................100 PSF
      2. STADIUM SEATING ....100 PSF
      3. MECHANICAL ROOMS.................80 PSF

   B. ROOF LOAD ....20 PSF

   C. SNOW LOADS
      1. GROUND SNOW LOAD: P_g=15 PSF
      2. FLAT–ROOF SNOW LOAD: P_f=16.5 PSF
      3. SNOW EXPOSURE FACTOR: C_e=1.0
      4. THERMAL FACTOR: C_t=1.0
      5. SNOW LOAD IMPORTANCE FACTOR: I_s = 1.1

   D. WIND LOADS
      1. BASIC WIND SPEED=120 MPH (ULTIMATE) (3–SECOND GUSTS)
      2. WIND IMPORTANCE FACTOR: I_w=1.0
      3. EXPOSURE CATEGORY: B
      4. DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING: P=23.0 PSF (WORKING STRESS)
      5. DESIGN WIND PRESSURE FOR MAIN WINDFORCE RESISTING SYSTEM: P=15.0 PSF (WORKING STRESS)
      6. INTERNAL PRESSURE COEFFICIENT: ±0.18

   E. EARTHQUAKE LOADS
      1. BUILDING OCCUPANCY CATEGORY IIISEISMIC USE GROUP: III
      2. MAPPED SPECTRAL RESPONSE ACCELERATION: S_s=0.229%, S_1=0.117%
      3. SEISMIC DESIGN CATEGORY B
      4. SITE CLASS C
      5. RESPONSE MODIFICATION FACTOR=4.0
      6. SYSTEM OVER STRENGTH FACTOR=2.5
      7. DEFLECTION AMPLIFICATION FACTOR=4.0
      8. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
      9. SPECTRAL RESPONSE COEFFICIENTS S_{DS}=0.183, S_{D1}=0.133 C_s=0.0572
      10. DESIGN BASE SHEAR =0.0572 W
      11. BASIC STRUCTURAL SYSTEM: ORDINARY REINFORCED CONCRETE SHEAR WALLS

CONCRETE

1. ALL CONCRETE FOR GENERAL USE (INCLUDING FOOTINGS, FOUNDATION WALLS AND ICF WALLS) SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI.

2. REINFORCING STEEL SHALL BE AS FOLLOWS:
   STIRRUPS AND TIES.....................ASTM A615 GRADE 60
   ALL OTHER REINFORCING............ASTM A615 GRADE 60
   WELDED WIRE FABRIC*................ASTM A185
   *WELDED WIRE FABRIC FOR USE IN ELEVATED SLABS ON METAL DECK SHALL BE SUPPLIED IN FLAT SHEETS, NOT ROLLS. ROLLS OF WELDED WIRE FABRIC PROVIDED FOR THIS PURPOSE WILL BE REJECTED AND RETURNED TO SUPPLIER.

3. PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH ACI DETAILING MANUAL. ALL BAR SUPPORTS IN AREAS WHERE CONCRETE WILL BE EXPOSED SHALL HAVE PLASTIC FEET. PRECAST CONCRETE (fc'=3000psi) BLOCKS 3"x3"x3" SHALL BE USED TO SUPPORT REINFORCING OFF OF THE GROUND. AT ALL OTHER LOCATIONS, CHAIRS OR STANDEES SHALL BE USED.

4. DETAILING, FABRICATION AND PLACING OF REINFORCING SHALL CONFORM TO APPLICABLE PROVISIONS OF ACI 315 AND ACI 318.

5. SLABS, FOUNDATION WALLS AND FOOTINGS SHALL HAVE NO HORIZONTAL JOINTS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL KEYED BULKHEADS. ALL REINFORCEMENT SHALL CONTINUE THROUGH JOINTS.

6. BEFORE PLACING CONCRETE, THE CONTRACTOR SHALL NOTIFY ALL SUBCONTRACTORS TO BE SURE ALL SLEEVES, CONDUIT, CHASES, ETC. ARE PROPERLY INSTALLED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AS SOON AS PRACTICAL, BUT AT LEAST 24 HOURS PRIOR TO PLACING CONCRETE TO ALLOW FOR INSPECTION OF REINFORCING AND EMBEDDED ITEMS.

7. MATERIALS SHALL COMPLY WITH REQUIREMENTS OF DESIGNATED SPECIFICATIONS OF AMERICAN SOCIETY FOR TESTING AND MATERIALS, 1916 RACE STREET, PHILADELPHIA, PENNSYLVANIA.

8. CONSTRUCTION PROCEDURES SHALL COMPLY WITH RECOMMENDATIONS SET FORTH IN DESIGNATED STANDARDS OF AMERICAN CONCRETE INSTITUTE, P.O. BOX 9094, FARMINGTON HILLS, MICHIGAN 48333.

9. ADMIXTURE OTHER THAN AIR–ENTRAINING SHALL NOT BE USED WITHOUT APPROVAL OF THE ARCHITECT/ENGINEER. AIR–ENTRAINING ADMIXTURES TO CONFORM TO ASTM C260.

10. CURING COMPOUND SHALL CONFORM TO FEDERAL SPECIFICATION TT–C800A, AND A.S.T.M. C309. THE MATERIAL SHALL BE EQUAL TO SONNEBORN KUR–N–SEAL, MASTERSEAL, BY MASTER BUILDERS, OR CLEAR SEAL, BY W.R. GRACE.

11. ALL REINFORCING SPLICES SHALL BE CLASS B TENSION LAP SPLICE.

12. SPREAD BARS AROUND SMALL OPENINGS AND SLEEVES IN SLABS AND WALLS WHERE POSSIBLE AND WHERE BAR SPACING WILL NOT EXCEED 1.5 TIMES THE NORMAL SPACING. DISCONTINUE BARS AT LARGE OPENINGS WHERE NECESSARY AND PROVIDE AN AREA OF REINFORCEMENT EQUAL TO THE INTERRUPTED REINFORCEMENT, DISTRIBUTING ONE–HALF OF THIS REINFORCEMENT EACH SIDE OF THE OPENING (CLASS B TENSION LAP SPLICED). HOLES LARGER THAN 12" IN ANY DIRECTION SHALL HAVE (1)#5x5'–0" DIAGONAL BAR IN BOTH FACES AT EACH CORNER.

13. PIER REINFORCEMENT SHALL BE DOWELED TO THE FOOTING. PROVIDE DOWELS EQUAL IN SIZE, NUMBER AND GRADE TO THE PIER REINFORCEMENT UNLESS OTHERWISE INDICATED. DOWELS SHALL BE HOOKED 90 DEGREES AT THE BOTTOM LEVEL OF FOOTING REINFORCEMENT. DOWELS SHALL BE LAPPED WITH THE PIER REINFORCEMENT.

14. PIER REINFORCEMENT SHALL BE THE SAME SIZE, NUMBER AND GRADE AS THE COLUMN/PILASTER REINFORCING, UNLESS OTHERWISE NOTED.

15. ALL VERTICAL CONCRETE SURFACES SHALL BE FORMED. HOWEVER, VERTICAL SURFACES OF FOOTINGS MAY BE EARTH–FORMED IF THE SOIL IS SUFFICIENTLY STIFF TO PREVENT CAVE–INS.

16. REINFORCING BARS SHALL BE IN PLACE AND SECURED PRIOR TO POURING CONCRETE.

17. REINFORCING BAR SHOP DRAWINGS SHALL SHOW NUMBER, SIZE AND LOCATION OF BARS, AS WELL AS LAP LENGTH AND CLEAR COVER.

18. ALL CONCRETE SLABS SUPPORTED BY SOIL OR GRANULAR SUB–BASE SHALL CONTAIN CONTROL JOINTS AND CONSTRUCTION JOINTS, AT SPACING NOT TO EXCEED 20 FEET ON CENTER IN BOTH DIRECTIONS. SAW–CUT JOINTS SHALL BE INSTALLED AS SOON AS THE CONCRETE IS HARD ENOUGH TO WITHSTAND SAWING WITHOUT RAVELING JOINT EDGES OR DISLODGING COARSE AGGREGATE PARTICLES. LIGHTWEIGHT EARLY–CUT SAWS SHALL BE USED. CONTRACTOR SHALL SUBMIT CONSTRUCTION AND CONTROL JOINT LAYOUT FOR APPROVAL PRIOR TO PLACING CONCRETE SLABS.

19. DOVETAIL SLOTS SHALL BE INSTALLED IN ALL CONCRETE WORK WHICH IS TO RECEIVE BRICK VENEER OR OTHER FACING MATERIALS.

FOUNDATIONS

1. ASSUMED ALLOWABLE SOIL BEARING PRESSURE:
   ISOLATED FOOTINGS.............2,500 PSF
   CONTINUOUS FOOTINGS.......2,500 PSF

2. FOUNDATIONS HAVE BEEN SIZED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL EXPLORATION REPORT (SOILS REPORT) PREPARED BY GREENBAUM ASSOCIATES, INC., DATED MARCH 9, 2009 (GREENBAUM PROJECT NO. 09–021).

3. THE CONTRACTOR SHALL THOROUGHLY FAMILIARIZE HIMSELF WITH THE EARTHWORK SPECIFICATIONS AND SHALL PERFORM ALL EARTHWORK OPERATIONS AND FOUNDATION INSTALLATION OPERATIONS IN ACCORDANCE WITH THESE SPECIFICATIONS.

4. PRIOR TO CONSTRUCTION OF ANY PERMANENT STRUCTURE, ALL EXISTING SURFACE FILL, ALL TOPSOIL AND ORGANIC MATERIAL, ALL WET, SOFT, LOOSE, OR UNDESIRABLE SOIL, AND ALL OLD ABANDONED CONCRETE SHALL BE REMOVED TO THE EXTENT REQUIRED BY THE EARTHWORK SPECIFICATIONS.

5. CONCRETE FOR FOOTINGS SHALL BE PLACED THE SAME DAY EXCAVATIONS ARE OPENED. IF THIS IS IMPOSSIBLE, STEPS SHALL BE TAKEN TO ADEQUATELY PROTECT THE OPEN EXCAVATION.

6. FOOTINGS AND SLABS ON GRADE SHALL BEAR ON FIRM NATURAL SOIL, OR ON PROPERLY COMPACTED ENGINEERED FILL, AS REQUIRED BY THE EARTHWORK SPECIFICATIONS. SOIL BENEATH FOOTINGS SHALL BE PROBED FOR ROCK TO VERIFY THAT THERE IS A MINIMUM SOIL DEPTH OF 6" BENEATH THE FOOTINGS. IF ROCK IS ENCOUNTERED, UNDERCUT & BACKFILL WITH CLAYEY SOIL PER TYPICAL DETAIL & SPECIFICATIONS.

7. ENGINEERED FILL & BACKFILL SHALL BE PLACED AND COMPACTED ACCORDING TO THE REQUIREMENTS OF THE SPECIFICATIONS.

8. THE OWNER WILL RETAIN A QUALIFIED GEOTECHNICAL ENGINEER TO INSPECT ALL BEARING SURFACES AND EARTHWORK OPERATIONS.

9. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED BY A GEOTECHNICAL ENGINEER BEFORE CONCRETE IS PLACED. THE ADEQUACY OF THE BEARING STRATUM SHALL BE DETERMINED BY THE GEOTECHNICAL ENGINEER. ALL GEOTECHNICAL FIELD REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER.

REINFORCED MASONRY

1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE F'M=1500 PSI.

2. GROUT FOR BOND BEAMS AND GROUTED CELLS IN CONCRETE MASONRY UNITS SHALL BE PEA GRAVEL CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI.

3. MORTAR FOR CONCRETE MASONRY SHALL BE TYPE S.

4. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCING IN ALL REINFORCED MASONRY WALLS AT 16" O.C. UNLESS NOTED OTHERWISE.

5. SPLICES IN VERTICAL REINFORCEMENT SHALL BE LAPPED IN ACCORDANCE WITH THE LAP SPLICE TABLE.

6. ALL MASONRY WALLS SHALL BE LATERALLY BRACED BY THE CONTRACTOR UNTIL ALL STRUCTURAL FRAMING AND DECKING HAVE BEEN INSTALLED IN UNITS OF CONSTRUCTION ADJACENT TO THE WALLS.

7. A BOND BEAM WITH (2)#5 BARS SHALL BE PROVIDED AT THE TOP OF ALL WALLS, AND AT THE BEARING ELEVATION OF STEEL JOISTS, UNLESS NOTED OTHERWISE.

8. ALL CMU UNITS LOCATED BELOW GRADE SHALL BE GROUTED SOLID.

9. AT BEAMS, COLUMNS AND LINTELS BEARING ON MASONRY WALLS, UNLESS DETAILED OR NOTED OTHERWISE, FILL TWO BLOCK CORES SOLID WITH GROUT AND REINFORCE EACH CORE WITH ONE #5 VERTICAL REBAR FULL HEIGHT OF WALL.

10. UNLESS OTHERWISE SHOWN OR NOTED, PROVIDE A 7–1/2"x7–1/2"x3/8" BEARING PLATE WITH (2)1/2" DIAMETER x 4" LONG HEADED STUDS EMBEDDED INTO GROUTED CORES AT ALL BEAMS BEARING ON MASONRY WALLS.

11. UNLESS OTHERWISE SHOWN OR NOTED, PLACE (1)#5 FULL–HEIGHT VERTICAL REINFORCING BAR AT ALL WALL CORNERS, ENDS OF WALLS,EACH SIDE OF CONTROL JOINTS, SIDES OF OPENINGS, AND WALL INTERSECTIONS. (PLACE (2)#5 BARS AT SIDES OF OPENINGS 10 FEET WIDE AND GREATER)

12. LOCATE VERTICAL CONTROL JOINTS IN ALL REINFORCED MASONRY WALLS AT A MAXIMUM SPACING OF 16'–0" O.C. LOCATIONS OF JOINTS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION. BOND BEAM AND REINFORCING SHALL BE CONTINUOUS THROUGH THE CONTROL JOINT.

13. SECURE ALL VERTICAL REINFORCING STEEL IN CMU WALLS WITH DUR–O–WALL REBAR POSITIONER OR APPROVED EQUAL.

14. GROUT FILL BEAM AND JOIST POCKETS IN CMU WALLS AFTER WELDS ARE INSPECTED.

15. CONTRACTOR SHALL SUBMIT DRAWINGS COORDINATED WITH THE MASONRY AND MEP CONTRACTORS, SHOWING THE MEP PENETRATIONS THROUGH LOAD BEARING WALLS. THESE DRAWINGS SHALL SHOW THE SIZE AND LOCATION OF ALL PENETRATIONS AND SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION

METAL FLOOR DECK

1. METAL FLOOR FORM DECKING SHALL BE FLAT ROLLED, GALVANIZED SHEETS OF STRUCTURAL QUALITY, MEETING THE REQUIREMENTS OF ASTM A653–94, GRADE 80. DECKING SHALL BE FACTORY GALVANIZED, COMPLYING WITH ASTM A653–94, COATING CLASS G90. CORRUGATION (PITCH x DEPTH) SHALL BE 2 1/2"x9/16" (MINIMUM) WITH GAUGE SUFFICIENT TO PROVIDE A MINIMUM SECTION MODULUS OF 0.039 IN.3. 26 GAUGE SHALL BE THE MINIMUM ALLOWED. DECKING SHALL BE FASTENED TO STEEL SUPPORTING MEMBERS WITH NO. 12 SELF–TAPPING METAL SCREWS AT 12"O.C., MAXIMUM SPACING. SIDE LAPS SHALL BE FASTENED TOGETHER WITH #10 SELF–TAPPING METAL SCREWS AT 12" O.C.

2. COMPOSITE METAL FLOOR DECKING SHALL BE FLAT ROLLED, GALVANIZED SHEETS OF STRUCTURAL QUALITY, MEETING THE REQUIREMENTS OF ASTM A653–94, STRUCTURAL GRADE. DECKING SHALL BE FACTORY GALVANIZED, COMPLYING WITH ASTM A653–94, COATING CLASS G90. THE DECK SHALL 1 1/2"x20 GAGE TYPE 1.5VLI20 AS MANUFACTURED BY VULCRAFT OR APPROVED EQUAL. DECKING SHALL BE FASTENED TO STEEL SUPPORTING MEMBERS WITH 5/8" DIAMETER PUDDLE WELD AT 12"O.C., MAXIMUM SPACING. SIDE LAPS SHALL BE WELDED AT 24"O.C. SHEAR STUDS MAY TAKE THE PLACE OF PUDDLE WELD.

3. ATTENTION IS CALLED TO THE FACT THAT THE METAL FLOOR DECK IS TO BE USED AS A PERMANENT FORM FOR THE CONCRETE FLOOR.

4. A TESTING COMPANY WILL BE RETAINED BY THE OWNER TO ENSURE THAT THE DECK IS FASTENED PROPERLY PRIOR TO PLACEMENT OF COVER MATERIALS. WRITTEN APPROVAL OF DECK INSTALLATION MUST BE SUBMITTED TO THE ARCHITECT/ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

5. ACCESSORIES SHALL BE STANDARD WITH THE MANUFACTURER AND SHALL BE FURNISHED AS NECESSARY TO COMPLETE THE FLOOR DECK INSTALLATION. ACCESSORIES SHALL INCLUDE EDGE CLOSURE PLATES FOR ALL OPEN EDGES OF DECK.

METAL ROOF DECK

1. 1 1/2" ROOF DECK SHALL BE 20 GAUGE GALVANIZED (G90) WITH A FABRICATED DEPTH OF 1 1/2" AND A VALLEY SPACING OF 6". DECKING SHALL BE FACTORY GALVANIZED. DECKING SHALL CONFORM TO ASTM A653–94 STRUCTURAL QUALITY GRADE 33 OR HIGHER. DECKING SHALL BE FASTENED TO STEEL SUPPORTING MEMBERS AT 12" O.C., MAXIMUM SPACING, WITH NO. 12 SELF–TAPPING METAL SCREW. SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED WITH NO. 10 METAL SCREWS, AT 12" O.C., MAXIMUM SPACING. SHEETS SHALL BE CONTINUOUS FOR AT LEAST THREE SPANS WHERE POSSIBLE.

2. COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS, EXCEPT AS OTHERWISE SHOWN OR SPECIFIED.
   A. A.I.S.I. "SPECIFICATIONS FOR THE DESIGN OF COLD–FORMED STEEL STRUCTURAL MEMBERS".
   B. AWS "STRUCTURAL WELDING CODE".
   C. SDI "STEEL DECK DESIGN MANUAL".

3. ACCESSORIES SHALL BE STANDARD WITH THE MANUFACTURER AND SHALL BE FURNISHED AS NECESSARY TO COMPLETE THE ROOF DECK INSTALLATION.

4. PROVIDE MANUFACTURER'S STANDARD 14 GA. GALVANIZED SUMP PANS FOR EACH ROOF DRAIN. COORDINATE TYPE AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.

5. ATTENTION IS CALLED TO THE FACT THAT THE METAL ROOF DECK IS DESIGNED FOR DIAPHRAGM ACTION. THEREFORE, ADDED CARE MUST BE TAKEN TO ASSURE PROPER INSTALLATION PROCEDURES ARE FOLLOWED.

6. A TESTING COMPANY WILL BE RETAINED BY THE OWNER TO ENSURE THAT THE DECK IS FASTENED PROPERLY PRIOR TO PLACEMENT OF COVER MATERIALS. WRITTEN APPROVAL OF DECK INSTALLATION MUST BE SUBMITTED TO THE ARCHITECT/ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

STRUCTURAL STEEL

1. ALL ROLLED STEEL PLATES, SHAPES (EXCLUDING WIDE FLANGE SHAPES), BARS AND MISCELLANEOUS ITEMS SHALL BE STRUCTURAL QUALITY CARBON STEEL COMPLYING WITH ASTM A36 (MINIMUM YIELD 36,000 PSI). WIDE FLANGE SHAPES SHALL BE STRUCTURAL QUALITY CARBON STEEL COMPLYING WITH ASTM A992 (MINIMUM YIELD 50,000 PSI).

2. TUBULAR STEEL MEMBERS (HSS) SHALL COMPLY WITH ASTM A500, GRADE B (MINIMUM YIELD 46,000 PSI).

3. STEEL PIPE COLUMNS(HSS) SHALL COMPLY WITH ASTM A500, GRADE B (MINIMUM YIELD 42000 PSI).

4. ALL BOLTED CONNECTIONS SHALL BE MADE WITH 3/4" DIAMETER ASTM F1852, TYPE 1 TWIST–OFF–TYPE TENSION–CONTROL BOLTS IN BEARING–TYPE CONNECTIONS.

5. ANCHOR RODS SHALL COMPLY WITH ASTM F1554, GRADE 36.

6. EXPANSION ANCHORS SHALL BE HILTI CARBON STEEL KWIK BOLT 3 ANCHOR MANUFACTURED BY HILTI FASTENING SYSTEMS, OR APPROVED EQUAL.

7. ADHESIVE ANCHORS SHALL CONSIST OF AN HAS–E STEEL ANCHOR ROD WITH THE HIT HY200 ADHESIVE (HIT HY70 ADHESIVE FOR MASONRY CONSTRUCTION WITH VOIDS) SUPPLIED BY HILTI FASTENING SYSTEMS, OR APPROVED EQUAL. INSTALL IN ACCORDANCE WITH THE SUPPLIER'S RECOMMENDATIONS.

8. WELDED HEADED STUDS TO BE USED AS CONCRETE ANCHORS OR SHEAR STUDS SHALL BE LOW CARBON STEEL SOLID FLUXED STUDS COMPLYING WITH ASTM A–108, WITH A MINIMUM Fu=60KSI. STUDS SHALL BE AUTOMATICALLY END WELDED. THE SPECIFIED LENGTH IS THE AFTER WELD LENGTH (AWL).

9. DEFORMED BAR ANCHORS (DBA): LOW CARBON STEEL PER ASTM A496(Fu=80KSI), SHALL BE AUTOMATICALLY END WELDED.

10. ALL WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED TO PERFORM EACH TYPE OF WELD REQUIRED. ALL WELDS AND WELDING PROCEDURES SHALL COMPLY WITH AWS D1.1, USING E70XX ELECTRODES UNLESS NOTED OTHERWISE.ALL WELDS SHALL BE INSPECTED.

11. WELD SIZES NOT SHOWN ON DESIGN DRAWINGS SHALL BE MINIMUM SIZE REQUIRED BY AWS D1.1 (LATEST EDITION) ACCORDING TO THE MATERIAL THICKNESS BEING WELDED. ALL WELDS SHALL BE PRE–QUALIFIED PER AWS D1.1 (LATEST EDITION).

12. STEEL FRAMEWORK SHALL NOT BE ASSUMED STRUCTURALLY STABLE UNTIL ALL MEMBERS ARE IN PLACE AND CONNECTIONS ARE INSTALLED. ANY USE OF THE PARTIALLY ERECTED FRAMEWORK FOR TEMPORARY SUPPORT OF ANY KIND SHALL BE DONE ONLY AT THE CONTRACTOR'S RISK.

13. COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS, EXCEPT AS OTHERWISE SHOWN OR SPECIFIED HEREIN.
    A. A.I.S.C. "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
    B. A.I.S.C. "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
    C. A.I.S.C. "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
    D. AWS "STRUCTURAL WELDING CODE."

14. ALL CONNECTIONS NOT INDICATED ON THE DESIGN DRAWINGS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE STRUCTURAL STEEL IS TO BE ERECTED, RETAINED BY THE STEEL FABRICATOR. ALL CALCULATIONS AND SHOP DRAWINGS SHALL BE DULY STAMPED AND SIGNED BY THE REGISTERED STRUCTURAL ENGINEER AND SUBMITTED FOR REVIEW BY THE ARCHITECT. STAMPING AND SIGNING OF SHOP DRAWINGS SHALL BE FOR THE EXCLUSIVE PURPOSE OF CERTIFYING THAT THE CONNECTIONS ARE DETAILED AS PER THE DESIGN PERFORMED BY THE REGISTERED STRUCTURAL ENGINEER. FAILURE TO SUBMIT STAMPED AND SIGNED CALCULATIONS AND STAMPED AND SIGNED SHOP DRAWINGS SHALL BE SUFFICIENT CAUSE FOR REJECTION OF SHOP DRAWINGS. THE CONTRACTOR SHALL BE LIABLE FOR THE DIMENSION, FIT, TOLERANCES, FABRICATION AND ERECTION.

15. SIMPLE SPAN CONNECTIONS FOR BEAMS SHALL CONSIST OF STANDARD DOUBLE–ANGLE BOLTED AND/OR WELDED CONNECTIONS, AND SHALL BE DESIGNED FOR ONE–HALF THE BEAM LOAD CAPACITY AS GIVEN IN AISC TABLE 3–6 "MAXIMUM TOTAL UNIFORM LOAD" (AISC MANUAL, 13TH EDITION).

16. LENGTH OF CONNECTION ANGLES FOR BEAM–TO–COLUMN OR BEAM–TO–BEAM CONNECTIONS SHALL BE THE LARGEST STANDARD LENGTH LESS THAN OR EQUAL TO THE "T" DIMENSION OF THE BEAM. STANDARD LENGTHS AND AVAILABLE STRENGTH OF CONNECTION ANGLES ARE FOUND IN "A.I.S.C. MANUAL OF STEEL CONSTRUCTION" (13TH EDITION), TABLES 10–1 THRU 10–3.

17. PROVIDE VERTICAL WEB STIFFENERS ON EACH SIDE OF WEB OF BEAM AT ALL POINTS SUBJECTED TO CONCENTRATED LOADS, SUCH AS COLUMN RESTING ON BEAM AND BEAM FRAMING INTO A BEAM. THE STIFFENERS SHALL EXTEND TO FULL DEPTH OF BEAM AND THE BOUNDARY OF FLANGE WITH MINIMUM THICKNESS OF 3/8".

18. ANY CAMBER EXISTING IN BEAMS SHALL BE TURNED POSITIVE UPWARD.

19. BURNING OF HOLES IN STRUCTURAL STEEL IS NOT PERMITTED WITHOUT PRIOR APPROVAL OF THE ENGINEER OF RECORD.

20. MAINTAIN WORK IN A SAFE AND STABLE CONDITION DURING ERECTION. PROVIDE TEMPORARY SHORING AND BRACING MEMBERS AS REQUIRED, WITH CONNECTIONS OF SUFFICIENT STRENGTH TO BEAR IMPOSED LOADS. REMOVE TEMPORARY MEMBERS AND CONNECTIONS WHEN PERMANENT MEMBERS ARE IN PLACE AND FINAL CONNECTIONS ARE MADE. PROVIDE TEMPORARY GUY LINES TO ACHIEVE PROPER ALIGNMENT AND STABILITY OF THE STRUCTURE AS ERECTION PROCEEDS.

21. MOMENT CONNECTIONS DESIGNATED BY ◀ SHALL BE DESIGNED BY THE STEEL SUPPLIER IN ACCORDANCE WITH AISC "MANUAL OF STEEL CONSTRUCTION". THE CONNECTION SHALL BE DESIGNED FOR THE MOMENT CAPACITY OF THE BEAM AND BEAM VERTICAL LOAD CAPACITY.

22. HIGH STRENGTH BOLTED CONNECTIONS AND WELDED CONNECTIONS SHALL BE INSPECTED BY AN INDEPENDENT TESTING LABORATORY. ALL COMPLETE–PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED BY A QUALIFIED INSPECTOR. INSPECTION AND TESTING WILL BE PAID FOR BY THE OWNER.

23. ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE HOT–DIPPED GALVANIZED.

24. THE STRUCTURAL STEEL SUPPLIER SHALL PROVIDE AND INSTALL SHIPS LADDERS AND STAIRS, AND ALL MISCELLANEOUS STEEL REQUIRED FOR PROPER INSTALLATION OF THE SHIPS LADDERS AND STAIRS. THE SHIPS LADDERS AND STAIRS SHALL BE DESIGNED BY THE STRUCTURAL STEEL SUPPLIER IN ACCORDANCE WITH THE 2013 KENTUCKY BUILDING CODE. SHOP DRAWING SHALL BE STAMPED & SIGNED BY A PROFESSIONAL ENGINEERED REGISTERED IN KENTUCKY, AND SUBMITTED FOR REVIEW.

SLESSER ENGINEERING, INC.
STRUCTURAL ENGINEERS
2325 LIME KILN LANE
LOUISVILLE, KENTUCKY 40222

STEEL JOISTS

1. STEEL JOISTS SHALL MEET ALL REQUIREMENTS OF THE S.J.I. AND A.I.S.C. STANDARD SPECIFICATIONS FOR THE TYPE AND SERIES SHOWN.

2. STEEL JOISTS SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH LATEST S.J.I. AND A.I.S.C. SPECIFICATIONS.

3. DESIGN OF STEEL JOISTS SHALL BE PERFORMED BY A REGISTERED STRUCTURAL ENGINEER AND SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE STEEL JOISTS AND THEIR CONNECTIONS.

4. STEEL JOISTS DESIGNATED "SPECIAL" (SPECIAL, NON–STANDARD) SHALL BE DESIGNED BY THE MANUFACTURER FOR THE LOADS INDICATED ON THE DRAWINGS. DESIGN SHALL CONFORM TO AISC AND SJI STANDARD SPECIFICATIONS AND SHALL BE PERFORMED BY A REGISTERED PROFESSIONAL ENGINEER.

5. STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS FOR A NET UPLIFT (DUE TO WIND LOADING) OF 10 PSF. DIAGONAL BRIDGING OR BRACING TO LATERALLY BRACE THE BOTTOM CHORD SHALL BE PROVIDED AS REQUIRED.

6. BRIDGING SHALL BE FURNISHED AND INSTALLED AS REQUIRED BY THE A.I.S.C. AND S.J.I. STANDARD SPECIFICATIONS AND/OR AS INDICATED ON PLANS. BRIDGE JOIST IMMEDIATELY AFTER ERECTION AND BEFORE CONSTRUCTION LOADS ARE APPLIED.

7. THE ENDS OF BRIDGING LINES TERMINATING AT MASONRY WALLS SHALL BE ANCHORED BY STRAP ANCHORS ATTACHED TO THE WALL UNLESS OTHERWISE SHOWN OR NOTED. GROUT JOIST POCKETS SOLID.

8. FURNISH AND INSTALL BOTTOM AND TOP CHORD LATERAL BRACING AS REQUIRED FOR STRENGTH AND STABILITY OF JOISTS.

9. ENDS OF STEEL JOISTS SHALL BE ANCHORED TO THE STEEL SUPPORTS BY WELDING.

10. EXTEND BOTTOM CHORD OF ALL JOISTS ON COLUMN LINES. WELD TO COLUMN AFTER ALL ROOF/FLOOR DEAD LOADS HAVE BEEN APPLIED.

11. PROVIDE ADDITIONAL L2X2X3/16 DIAGONALS AND FIELD WELD AT ALL POINTS WHERE EQUIPMENT IS SUPPORTED ON OR HUNG FROM THE CHORDS OF THE JOISTS. THE ANGLE SHALL EXTEND FROM THE POINT OF LOAD APPLICATION TO THE CLOSEST PANEL POINT IN THE OPPOSITE CHORD MEMBER. SEE TYPICAL DETAIL.

12. PROVIDE MISC. ANGLE FRAMING BETWEEN JOISTS AS REQUIRED AT ALL ROOF DRAINS AND MISC. FLOOR AND ROOF PENETRATIONS. SEE TYPICAL DETAIL.

13. PRIOR TO FABRICATION, SHOP DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE JOIST LAYOUT, ERECTION DETAILS, CONNECTION DETAILS, BRIDGING DETAILS, MARK, TYPE AND LOCATION.

14. ALL STEEL JOISTS SHALL BE PRODUCED BY AN S.J.I. MEMBER OR, IF PRODUCER IS NOT AN S.J.I. MEMBER, SHOP DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF AN ENGINEER REGISTERED IN THE STATE WHERE THE JOISTS WILL BE ERECTED, WHO SHALL CERTIFY THAT THE JOISTS ARE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE A.I.S.C. AND S.J.I. SPECIFICATIONS.

15. DESIGN JOIST TOP CHORD EXTENSIONS AND EXTENDED ENDS FOR DEAD LOADS AND LIVE LOADS OF BACK SPAN.

MISCELLANEOUS

1. SHRINKAGE–RESISTING GROUT FOR USE BENEATH COLUMN BASE PLATES AND BEAM BEARINGS SHALL BE PRE–MIXED, FACTORY PACKAGED, NON–STAINING, NON–METALLIC, NON–GASING MORTAR GROUTING COMPOUND, COMPLYING WITH THE REQUIREMENTS OF A.S.T.M. C1107. GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6,000 PSI.

2. MATERIAL FOR USE AS VAPOR BARRIER BENEATH CONCRETE SLABS ON GRADE SHALL BE 10 MIL POLYETHYLENE SHEETS, BLACK IN COLOR, COMPLYING WITH ASTM D–2103. SHEETS SHALL BE LAPPED A MINIMUM OF 6" AT ALL EDGES. SPECIAL CARE SHALL BE TAKEN TO PREVENT PUNCTURING SHEETS PRIOR TO PLACEMENT OF SLABS.

3. NO CHANGE IN SIZE OF STRUCTURAL ELEMENTS OR MODIFICATION THEREOF SHALL BE MADE, NOR ARE ANY OPENINGS OR SLEEVES THROUGH ANY STRUCTURAL ELEMENTS PERMITTED, UNLESS DETAILED ON THE DRAWINGS.

4. CONSULT ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION, SIZES AND EXTENT OF CHASES, INSERTS, RECESSES, REGELETS, FINISHES, DEPRESSIONS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

5. SPECIAL STRUCTURAL INSPECTIONS ARE REQUIRED ON THIS PROJECT IN ACCORDANCE WITH THE KENTUCKY BUILDING CODE. THE OWNER WILL BEAR THE COSTS OF THE SPECIAL STRUCTURAL INSPECTIONS. THE CONTRACTOR SHALL COORDINATE HIS WORK AND SCHEDULING, AND SHALL COOPERATE FULLY WITH THE AGENCIES PERFORMING THE SPECIAL STRUCTURAL INSPECTIONS.

6. ALL WELDED WIRE FABRIC IN SLABS ON GRADE AND ELEVATED SLABS SHALL BE SUPPORTED BY CHAIRS, BOLSTERS, OR OTHER APPROVED SUPPORTING DEVICES. "PULLING–UP" OF MESH AFTER CONCRETE HAS BEEN PLACED IS NOT ACCEPTABLE.

CONTRACTOR RESPONSIBILITIES

1. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.

2. COORDINATE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.

3. VERIFY THE DIMENSIONS, ELEVATIONS AND SITE CONDITIONS BEFORE STARTING WORK. ANY DISCREPANCY BETWEEN SUCH DETAILS AND DIMENSIONS AS MAY OCCUR SHALL BE REPORTED TO THE ARCHITECT/ ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH THE WORK.

4. NOTIFY, IN WRITING, THE STRUCTURAL ENGINEER OF CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN IN THE STRUCTURAL DOCUMENTS.

5. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

6. CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC.

7. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA SAFETY REGULATIONS.

8. SUBMITTALS

   FURNISH TWO PRINTS OF SHOP DRAWINGS. FURNISH THREE COPIES OF OTHER STRUCTURAL SUBMITTALS.

   REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

studio
studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

STATE OF KENTUCKY
CARL J. SLESSER
#10694
LICENSED PROFESSIONAL ENGINEER
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

GENERAL NOTES

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

DESIGNED TO EARN THE ENERGY STAR

BG 15-229

DATE: 12/08/2015
DRAWN BY: JEK
CHECKED BY: TEL
REVISIONS:

2011-02

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SPECIAL STRUCTURAL INSPECTIONS

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL STRUCTURAL INSPECTIONS DURING CONSTRUCTION, IN ACCORDANCE WITH SECTION 1704 OF THE 2013 KENTUCKY BUILDING CODE AND THE 2012 INTERNATIONAL BUILDING CODE.

2. MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS ARE AS FOLLOWS:

   A. CONCRETE (INCLUDING ICF CONCRETE)
      1. THE MATERIALS, SIZE, LOCATION AND INSTALLATION DETAILS OF REINFORCING STEEL SHALL BE INSPECTED FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND ACI 318.
      2. CONCRETE PLACING METHODS AND CURING OPERATIONS SHALL BE OBSERVED BY THE SPECIAL INSPECTOR FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS, ACI 318 AND ACI 301.
      3. FORMS FOR CONCRETE SHALL BE INSPECTED FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND ACI 318.
      4. VERIFY USE OF THE REQUIRED DESIGN MIX.
      5. SAMPLE FRESH CONCRETE TO FABRICATE SPECIMENS FOR STRENGTH TESTS, TO PERFORM SLUMP AND AIR CONTENT TESTS, AND TO DETERMINE THE TEMPERATURE OF THE CONCRETE. VERIFY COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND ACI 318.
      6. COMPLY WITH THE REQUIREMENTS OF TABLE 1, THIS SHEET.

   B. STRUCTURAL STEEL
      1. VERIFY THAT THE STEEL FABRICATOR MAINTAINS AND PROPERLY IMPLEMENTS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES.
      2. SPECIAL INSPECTION OF FABRICATED STRUCTURAL STEEL AND THE FABRICATION PROCESS SHALL BE PERFORMED ON THE PREMISES OF THE FABRICATOR'S SHOP. IN-PLANT INSPECTIONS ARE NOT REQUIRED IF THE FABRICATOR IS CERTIFIED BY THE AISC.
      3. BOLTS, NUTS AND WASHERS SHALL BE INSPECTED TO CONFIRM IDENTIFICATION MARKINGS AND CONFORMANCE TO ASTM STANDARDS SPECIFIED BY THE DESIGN ENGINEER. MANUFACTURER'S CERTIFICATE OF COMPLIANCE IS REQUIRED.
      4. STRUCTURAL STEEL MEMBERS SHALL BE INSPECTED TO CONFIRM MATERIAL IDENTIFICATION MARKINGS AND CONFORMANCE TO ASTM STANDARDS SPECIFIED IN THE CONTRACT DOCUMENTS. PROVIDE MILL CERTIFICATION TEST REPORTS.
      5. WELD FILLER MATERIALS SHALL BE INSPECTED TO CONFIRM CONFORMANCE TO AWS D1.1. MANUFACTURER'S CERTIFICATE OF COMPLIANCE IS REQUIRED.
      6. EACH COMPLETE PENETRATION GROOVE WELD (SHOP WELD OR FIELD WELD) SHALL BE TESTED FOR THE FULL LENGTH OF THE WELD BY ULTRASONIC TESTING.
      7. EACH FILLET WELD SHALL BE VISUALLY INSPECTED.
      8. EACH HIGH STRENGTH BOLT SHALL BE INSPECTED TO CONFIRM THAT THE SPECIFIED TENSION OR SNUGNESS HAS BEEN ACHIEVED.
      9. THE SPECIAL INSPECTOR SHALL PERFORM AN INSPECTION OF THE STEEL FRAME TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONTRACT DOCUMENTS, SUCH AS BRACING, STIFFENING, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.
      10. VERIFY THAT ATTACHMENT OF ROOF DECK AND FLOOR DECK CONFORMS TO THE CONTRACT DOCUMENTS.
      11. WELDS OF HEADED SHEAR STUDS TO STEEL BEAMS SHALL BE FIELD-TESTED FOR ADEQUACY.
      12. COMPLY WITH THE REQUIREMENTS OF TABLE 2, THIS SHEET.

   C. SOILS
      1. DETERMINE THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE SOILS REPORT.
      2. FILL PLACEMENT SHALL BE OBSERVED. VERIFY THAT IN-PLACE DENSITY, PROPER MATERIALS AND LIFT THICKNESS ARE IN ACCORDANCE WITH THE SOILS REPORT.
      3. FOR SPREAD FOOTINGS BEARING ON SOIL, DETERMINE THAT THE SOIL IS SATISFACTORY TO SUPPORT THE DESIGN PRESSURES INDICATED IN THE SOILS REPORT AND ON THE DRAWINGS.
      4. COMPLY WITH REQUIREMENTS OF TABLE 4, THIS SHEET.

   D. REINFORCED CONCRETE MASONRY
      1. VERIFY PROPORTIONS OF SITE-PREPARED MORTAR AND GROUT.
      2. VERIFY CONSTRUCTION OF MORTAR JOINTS.
      3. VERIFY THAT THE SIZE, GRADE AND TYPE OF CONCRETE MASONRY UNITS COMPLY WITH THE PROVISIONS OF THE CONSTRUCTION DOCUMENTS.
      4. VERIFY LOCATION OF REINFORCEMENT AND CONNECTORS, AND CLEANLINESS OF GROUT SPACE. VERIFY SIZE, GRADE AND TYPE OF REINFORCEMENT.
      5. VERIFY TYPE, SIZE AND LOCATIONS OF ANCHORS AND EMBEDDED PLATES, INCLUDING CONNECTION DETAILS TO OTHER STRUCTURAL MEMBERS.
      6. VERIFY ADEQUATE PROTECTION OF MASONRY DURING COLD AND/OR HOT WEATHER.
      7. VERIFY PLACEMENT OF GROUT TO ENSURE COMPLIANCE WITH CONSTRUCTION DOCUMENTS.
      8. PREPARE, OR OBSERVE THE PREPARATION OF, ALL REQUIRED MORTAR AND GROUT SPECIMENS FOR TESTING.
      9. VERIFY THAT VERTICAL CONTROL JOINTS ARE PROPERLY INSTALLED AND CORRECTLY LOCATED.
      10. COMPLY WITH THE REQUIREMENTS OF TABLE 3, THIS SHEET.

   E. STEEL JOISTS
      1. VERIFY SIZE, LOCATIONS AND SPACINGS OF STEEL BAR JOISTS.
      2. VERIFY THAT BEARING AND CONNECTION DETAILS ARE IN ACCORCANCE WITH THE APPROVED SHOP DRAWINGS AND CONTRACT DOCUMENTS.

   F. METAL ROOF DECK
      1. VERIFY THAT PROPER DECK IS USED.
      2. VERIFY THAT CONNECTIONS OF THE METAL DECK TO THE STRUCTURAL MEMBERS ARE IN ACCORDANCE WITH THE DESIGN DRAWINGS AND THE APPROVED SHOP DRAWINGS.

   G. METAL FLOOR DECK
      1. VERIFY THAT PROPER DECK IS USED.
      2. VERIFY THAT CONNECTIONS OF THE METAL DECK TO THE STRUCTURAL MEMBERS ARE IN ACCORDANCE WITH THE DESIGN DRAWINGS AND THE APPROVED SHOP DRAWINGS.

3. CERTIFICATES OF COMPLIANCE (INSPECTION REPORTS) SHALL BE SUBMITTED WEEKLY BY THE SPECIAL INSPECTOR TO THE ENGINEER OF RECORD. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE ENGINEER OF RECORD PRIOR TO COMPLETION OF THAT PHASE OF THE WORK.

4. AT THE COMPLETION OF THE TESTING AND SPECIAL INSPECTIONS, THE LICENSED PROFESSIONAL ENGINEER IN CHARGE OF PERFORMING THE SPECIAL INSPECTIONS SHALL SUBMIT TO THE ENGINEER OF RECORD A FINAL REPORT OF SPECIAL INSPECTIONS, AFFIXED WITH HIS/HER SEAL, DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS.

5. ADMINISTRATIVE AND PROCEDURAL REQUIREMENTS FOR SPECIAL STRUCTURAL INSPECTIONS AND TESTS ARE INDICATED IN SPECIFICATION SECTION 01411.

TABLE 1 — REQUIRED VERIFICATION & INSPECTION OF CONCRETE CONSTRUCTION

SCASP001-14

INSPECTION TASK	FREQUENCY* Continuous	FREQUENCY* Periodic	REFERENCE FOR CRITERIA: REFERENCED STANDARD	REFERENCE FOR CRITERIA: IBC REFERENCE
1. Inspection of reinforcing steel, including prestressing tendons, and placement		X	ACI 318: 3.5, 7.1–7.7	1910.4
2. Inspection of reinforcing steel welding:			AWS D1.4 ACI 318: 3.5.2	
a. Verification of weldability of reinforcing steel other than ASTM A706		X		
b. Reinforcing steel resisting flexural and axial forces intermediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcing	X			
c. Shear reinforcement	X			
d. Other reinforcing steel		X		
3. Inspect bolts to be installed in concrete prior to and during placement of concrete		X	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1
4. Inspection of anchors post-installed in hardened concrete members		X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1
5. Verifying use of required design mix		X	ACI 318: CH. 4, 5.2–5.4	1904.2. 1910.2, 1910.3
6. Sampling fresh concrete and performing slump, air content and determining the temperature of fresh concrete at the time of making specimens for strength of tests	X		ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
7. Inspection of concrete and shotcrete placement for proper application techniques	X		ACI 318: 5.9, 5.10	1910.6, 1910.7 1910.8
8. Inspection for maintenance of specified curing temperature and techniques		X	ACI 318: 5.11–5.13	1910.9
9. Inspection of prestressed concrete:			ACI 318: 18.20 ACI 318: 18.18.4	
a. Application of prestressing forces.	X			
b. Grouting of bonded prestressing tendons in the seismic-force-resisting system	X			
10. Erection of precast concrete members		X	ACI 318: Ch. 16	
11. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs		X	ACI 318: 6.2	
12. Inspect formwork for shape, location and dimensions of the concrete member being formed		X	ACI 318: 6.1.1	

* The inspector shall monitor all of the tasks indicated. Under the heading "Periodic", the inspector shall be present a minimum of 25% of the time, monitoring the task while it is being performed. Frequency of "periodically" inspection shall be scheduled at approximately equal intervals throughout the duration of the task. Under the heading "Continuous", the inspector shall be present 100% of the time, monitoring the task while being performed.

TABLE 2 — REQUIRED VERIFICATION & INSPECTION OF STRUCTURAL STEEL

SCASP002-14

VERIFICATION AND INSPECTION	FREQUENCY* Continuous	FREQUENCY* Periodic	REFERENCE FOR CRITERIA: REFERENCED STANDARD
1. Material verification of high-strength bolt, nuts and washers:			Applicable ASTM material specifications; AISC 360, Section A3.3
a. Identification of markings to conform to ASTM standards specified in the approved construction. documents.		X	
b. Manufacturer's certificate of compliance required.		X	
2. Inspection of high-strength bolting:			AISC 360 Section M2.5
a. Bearing-type connections.		X	
b. Slip critical connections.	X		
3. Material verification of structural steel:			ASTM A6 or ASTM A568
a. Identification of markings to conform to ASTM standards specified in the approved construction. documents.		X	
b. Manufacturer's certified mill test reports.		X	
4. Material verification of weld filler materials:			AISC 360 Section A3.5
a. Identification of markings to conform to AWS specifications in the approved construction documents.		X	
b. Manufacturer's certificate of compliance required		X	
5. Inspection of welding:			AWS D1.1
a. Complete and partial penetration groove welds.	X		
b. Multi-pass fillet welds	X		
c. Single-pass fillet welds > $5/16$"	X		
d. Single-pass fillet welds ≤ $5/16$"		X	
e. Floor and Roof deck welds		X	AWS D1.3
6. Inspection of steel frame joint details for compliance with approved construction documents.			
a. Details such as bracing and stiffening		X	
b. Member locations		X	
c. Application of joint details at each connection.		X	
7. Additional bolt & welding inspection per AISC 360-10			AISC 360 Section N

* The inspector shall monitor all of the tasks indicated. Under the heading "Periodic", the inspector shall be present a minimum of 25% of the time, monitoring the task while it is being performed. Frequency of "periodically" inspection shall be scheduled at approximately equal intervals throughout the duration of the task. Under the heading "Continuous", the inspector shall be present 100% of the time, monitoring the task while being performed.

LEGEND & ABBREVIATIONS

- TYP = TYPICAL
- N.S. = NEAR SIDE
- F.S. = FAR SIDE
- O.C. = ON CENTER
- CMU = CONCRETE MASONRY UNIT
- T/ = TOP OF
- CLR = CLEAR
- U.N.O.= UNLESS NOTED OTHERWISE
- T&G = TONGUE AND GROOVE
- P.T. = PRESSURE TREATED
- HKB = HILTI KWIK BOLT 3
- CEA = CONCRETE EXPANSION ANCHOR
- LVL = LAMINATED VENEER LUMBER
- PSL = PARALLEL STRAND LUMBER
- GT = GIRDER TRUSS
- ALT = ALTERNATE
- EA = EACH
- W/ = WITH
- CLR COV = CLEAR COVER
- (E) = EXISTING
- DBA = DEFORMED BAR ANCHOR

CONCRETE PROTECTION FOR REINFORCEMENT IN CAST-IN-PLACE CONCRETE

THE FOLLOWING CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT; UNLESS NOTED OTHERWISE

	COVER, IN.
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
CONCRETE EXPOSED TO EARTH OR WEATHER	
#6 THROUGH #18 BARS	2
#5 BAR, W31 OR D31 WIRE, AND SMALLER	1 1/2
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	
SLABS, WALLS, JOIST:	
#14 AND #18 BARS	1 1/2
#11 BAR AND SMALLER	3/4
BEAMS AND COLUMNS:	
PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2

CFED007A

TABLE 3B — REQUIRED VERIFICATION & INSPECTION OF MASONRY CONSTRUCTION (LEVEL B QUALITY ASSURANCE)

SCASP003B-14

Verification of Slump flow and Visual Stability Index (VSI) as delivered to project the site in accordance with Article 1.5 B.1.b.3 for self-consolidating grout.

Verification of f'_m and f'_{AAC} in accordance with Article 1.4 B prior to construction, except where specifically exempted by the Code.

INSPECTION TASK	FREQUENCY* Continuous	FREQUENCY* Periodic	REFERENCE FOR CRITERIA: TMS 402/ ACI 530/ ASCE 5	REFERENCE FOR CRITERIA: TMS 602/ ACI 530.1/ ASCE 6
1. Verify compliance with the approved submittals		X		Art. 1.5
2. As masonry construction begins, verify that the following are in compliance:				
a. Proportions of site-prepared mortar		X		Art. 2.1, 2.6 A
b. Construction of mortar joints		X		Art. 3.3 B
c. Grade and size of prestressing tendons and anchorages		X		Art. 2.4 B, 2.4 H
d. Location of reinforcement, connectors, and prestressing tendons and anchorages		X		Art. 3.4, 3.6 A
e. Prestressing technique		X		Art. 3.6 B
f. Properties of thin-bed mortar for ACC masonry	X**	X***		Art. 2.1 C
3. Prior to grouting, verify that the following are in compilance:				
a. Grout space		X		Art. 3.2 D, 3.2 F
b. Grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages		X	Sec. 1.16	Art. 2.4, 3.4
c. Placement of reinforcement, connectors, and prestressing tendons and anchorages		X	Sec. 1.16	Art. 3.2 E, 3.4, 3.6 A
d. Proportions of site-prepared grout and prestressing grout for bonded tendons		X		Art. 2.6 B, 2.4 G.1.b
e. Construction of mortar joints		X		Art. 3.3 B
4. Verify during construction:				
a. Size and location of structural elements		X		Art. 3.3 F
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction		X	Sec. 1.16.4.3, 1.17.1	
c. Welding of reinforcement		X	Sec. 2.1.7.7.2, 3.3.3.4 (c), 8.3.3.4 (b)	
d. Preparation construction, and protectection of masonry during cold weather (temperature below 40°F (4.4°C)) or hot weather (temperature above 90°F (32.2°C))		X		Art. 1.8 C, 1.8 D
e. Application and measurement of prestressing force	X			Art. 3.6 B
f. Placement of grout and prestressing grout for bonded tendons is in compliance	X			Art. 3.5, 3.6 C
g. Placement of AAC masonry units and construction of thin-bed mortar joints	X**	X***		Art. 3.3 B.8
5. Observe preparation of grout specimens, mortar specimens, and/or prisms		X		Art. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4

* The inspector shall monitor all of the tasks indicated. Under the heading "Periodic", the inspector shall be present a minimum of 25% of the time, monitoring the task while it is being performed. Frequency of "periodically" inspection shall be scheduled at approximately equal intervals throughout the duration of the task. Under the heading "Continuous", the inspector shall be present 100% of the time, monitoring the task while being performed.

** Required for the first 5000 square feet (465 square meters) of AAC masonry.

*** Required after the first 5000 square feet (465 square meters) of AAC masonry.

TABLE 4 — REQUIRED VERIFICATIONS & INSPECTIONS & TESTS OF SOILS

SCASP004-14

VERIFICATION AND INSPECTION	FREQUENCY* Continuous	FREQUENCY* Periodic
1. Verify materials below shallow footings are adequate to achieve the design bearing capacity.		X
2. Verify excavations are extended to proper depth and have reached proper material.		X
3. Perform classification and testing of compacted filled materials.		X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.		X

* The inspector shall monitor all of the tasks indicated. Under the heading "Periodic", the inspector shall be present a minimum of 25% of the time, monitoring the task while it is being performed. Frequency of "periodically" inspection shall be scheduled at approximately equal intervals throughout the duration of the task. Under the heading "Continuous", the inspector shall be present 100% of the time, monitoring the task while being performed.

REINFORCEMENT LAP SPLICE LENGTH
(GRADE 60 REINFORCEMENT)

THE FOLLOWING LAP SPLICE SHALL BE PROVIDED FOR REINFORCEMENT; UNLESS NOTED OTHERWISE

	4000 PSI CONCRETE: TOP BARS	4000 PSI CONCRETE: OTHERS	MASONRY
#3	24"	19"	18"
#4	32"	25"	24"
#5	40"	31"	30"
#6	48"	37"	36"
#7	70"	54"	42"
#8	80"	62"	48"
#9	91"	70"	55"
#10	102"	79"	--
#11	113"	87"	--

(TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 IN. OF CONCRETE CAST BELOW THE BARS)

CFED007Q

NOTE: ALL WELDED WIRE FABRIC LAP SPLICES SHALL BE ONE SPACE PLUS 2" (i.e. WWF 6x6 = 8" SPLICE)

MISCELLANEOUS LINTEL (ML) SCHEDULE (SRES009R)
(FOR LINTELS NOT OTHERWISE SHOWN OR NOTED)

BLOCK LINTELS – 8" BEARING EACH END
3'–0" WIDE AND LESS USE 8" DP. BOND BEAM WITH (2) #5 BOT.
3'–1" WIDE TO 7'–0" WIDE USE 16" DP. BOND BEAM WITH (2) #5 TOP & BOTTOM.
7'–1" WIDE TO 11'–0" WIDE USE 16" DP. BOND BEAM WITH (2) #6 TOP & BOT.

STEEL LINTELS
PROVIDE ONE ANGLE FOR EVERY FOUR INCHES OF WIDTH

8" WALL = 2 ANGLES
12" WALL = 3 ANGLES
14" WALL = 3 ANGLES

4'–0" WIDE AND LESS USE ANGLE $4x3\frac{1}{2}x\frac{5}{16}$ W/ 8" BRG. EA. END
4'–1" WIDE TO 6'–0" USE ANGLE $5x3\frac{1}{2}x\frac{5}{16}$ W/ 8" BRG. EA. END
6'–1" WIDE TO 9'–0" USE ANGLE $6x3\frac{1}{2}x\frac{5}{16}$ W/ 8" BRG. EA. END
9'–1" WIDE TO 11'–0" USE ANGLE $6x3\frac{1}{2}x\frac{3}{8}$ W/ 12" BRG. EA. END

PROVIDE STAINLESS OR GALVANIZED STEEL WHERE LINTELS ARE EXPOSED TO WEATHER, COORDINATE WITH ARCHITECTURAL DRAWINGS & DETAILS

SLESSER ENGINEERING, INC.
STRUCTURAL ENGINEERS
2325 LIME KILN LANE
LOUISVILLE, KENTUCKY 40222

studio
studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

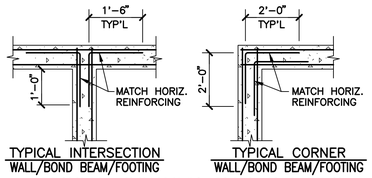
STATE OF KENTUCKY — CARL J. SLESSER #10694 — LICENSED PROFESSIONAL ENGINEER
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

SPECIAL INSPECTIONS & SCHEDULES
NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
DESIGNED TO EARN THE ENERGY STAR
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

BG 15-229
DATE: 12/08/2015
DRAWN BY: JEK
CHECKED BY: TEL
REVISIONS:

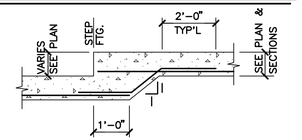
2011-02
PHASE - 3
S0.2
HS



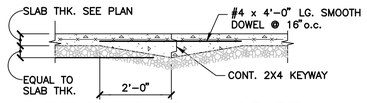
TYPICAL INTERSECTION WALL/BOND BEAM/FOOTING

TYPICAL CORNER WALL/BOND BEAM/FOOTING

TYPICAL CORNER BAR DETAILS



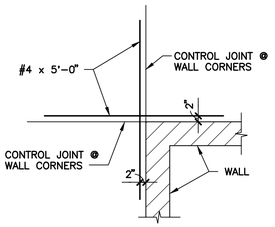
TYPICAL STEP FTG. DETAIL



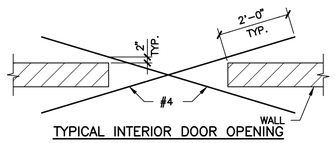
TYPICAL SLAB ON GRADE CONSTRUCTION JOINT DETAIL

TYPICAL SLAB ON GRADE CONTROL JOINT DETAIL

CFDS003B



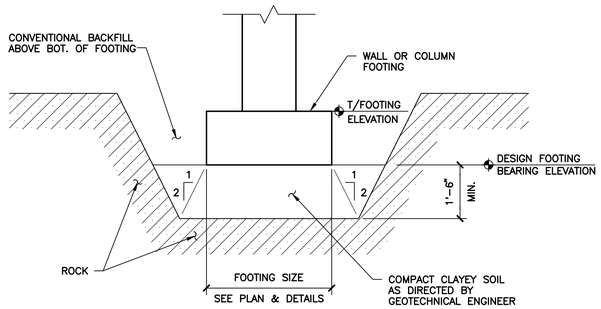
TYPICAL WALL CORNER



TYPICAL INTERIOR DOOR OPENING

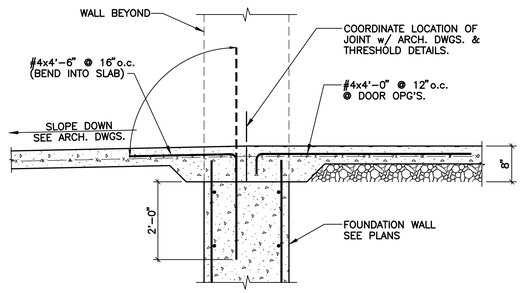
TYPICAL SLAB REINFORCING DETAILS

CFDP067



ROCK UNDERCUT AT FOOTING BEARING

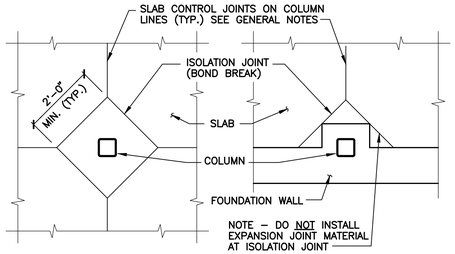
NOTE: ABOVE DETAIL REQUIRED IF ROCK IS ENCOUNTERED WITHIN 6" OF DESIGN BEARING ELEVATIONS



TYP. SECTION @ THRESHOLD

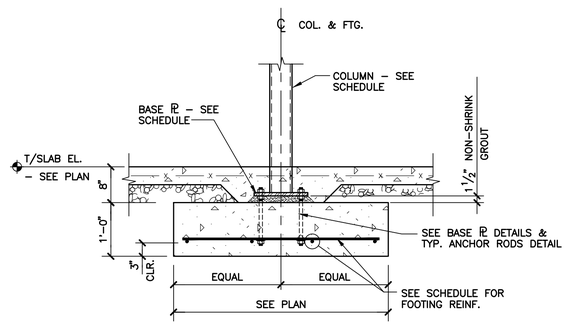
MLES027A A S0.3

SCALE: N.T.S.



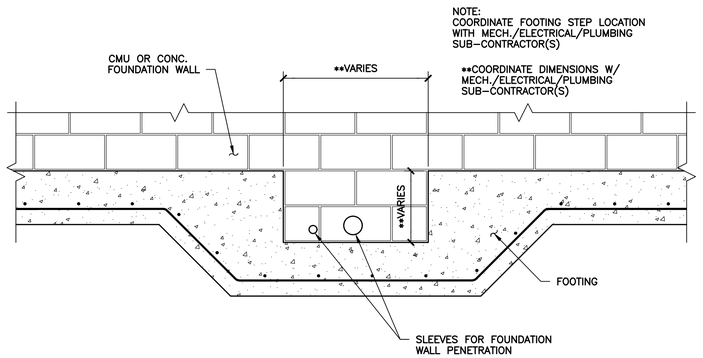
TYPICAL ISOLATION JOINT IN SLAB ON GRADE AT COLUMN

NO SCALE CFDD018G



TYPICAL INTERIOR COLUMN FOOTING DETAIL

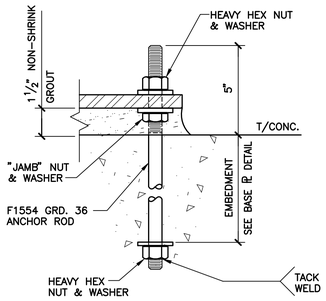
SCALE: 3/4" = 1'-0 CFES2093



TYPICAL STEP FOOTING DETAIL @ M E P PENETRATION

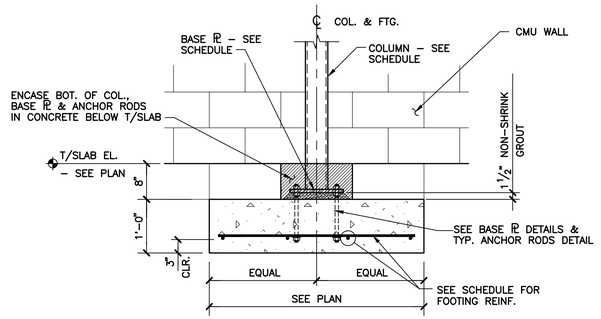
1. GENERAL CONTRACTOR SHALL COORDINATE ALL INTERIOR ISOLATED & STRIP FOOTING STEPS WITH M E P LAYOUT AND SUB-CONTRACTOR(S).

CFES1094



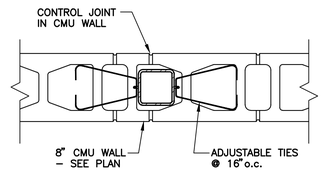
TYPICAL ANCHOR ROD DETAIL

SCALE: 3" = 1'-0 SBHD012C



TYPICAL INTERIOR COLUMN IN CMU WALL DETAIL

SCALE: 3/4" = 1'-0 CFES2093E



TYP. COL IN CMU DETAIL

SCALE: 1½" = 1'-0 MOCD025A

SLESSER ENGINEERING, INC.
STRUCTURAL ENGINEERS
2325 LIME KILN LANE
LOUISVILLE, KENTUCKY 40222

studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

STATE OF KENTUCKY CARL J. SLESSER #10994 LICENSED PROFESSIONAL ENGINEER 12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

TYPICAL DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

DESIGNED TO EARN THE ENERGY STAR

BG 15-229

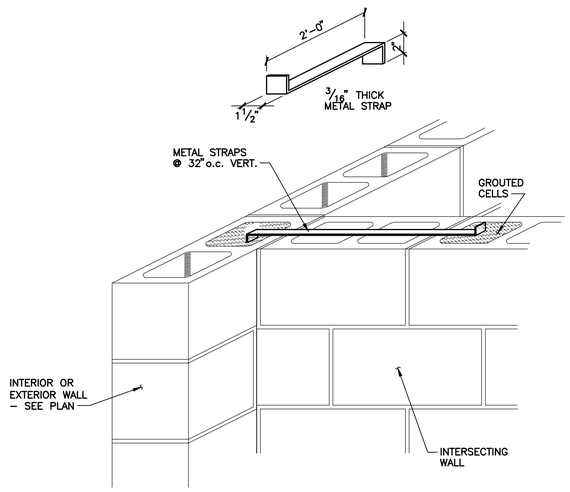
DATE: 12/08/2015
DRAWN BY: JEK
CHECKED BY: TEL
REVISIONS:

2011-02

PHASE - 3

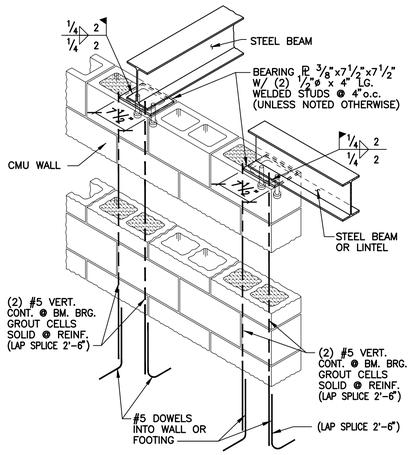
S0.3

HS



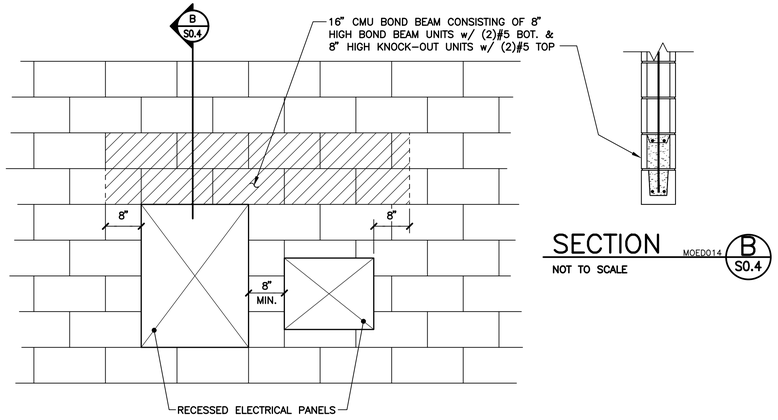
METAL STRAP DETAIL

N.T.S. @ INTERSECTING CMU WALLS PROTO14-E — A S0.4



TYPICAL DETAIL @ BEAM BRG.

SCALE: 3/4" = 1'-0 (UNLESS OTHERWISE NOTED) MOED002X

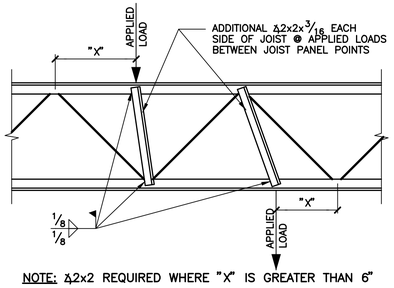


SECTION

NOT TO SCALE MOED014 — B S0.4

TYPICAL LINTEL DETAIL OVER ELECTRICAL PANELS RECESSED IN CMU WALLS

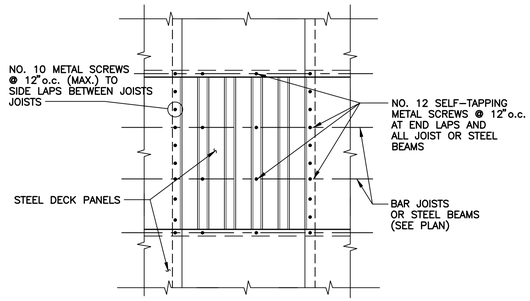
(SEE ELECTRICAL DRAWINGS FOR LOCATIONS OF PANELS)



NOTE: ∆2x2 REQUIRED WHERE "X" IS GREATER THAN 6"

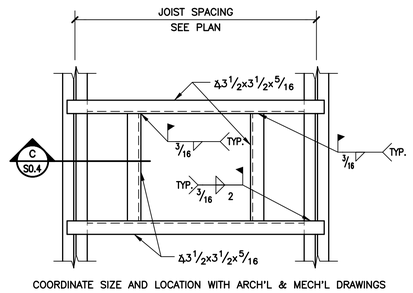
TYPICAL STEEL JOIST CHORD STIFFENER @ APPLIED LOADS

SCALE: NONE SRES008F



TYPICAL FASTENING PATTERN FOR STEEL DECK PANELS

SRED003A

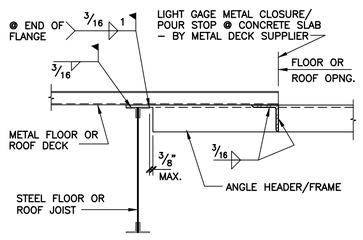


COORDINATE SIZE AND LOCATION WITH ARCH'L & MECH'L DRAWINGS

TYPICAL ROOF AND FLOOR OPENING FRAME

SRED003M

APPLICABLE TO ROOF DRAINS, MECHANICAL UNIT OPENINGS, AND OTHER ARCH'L/MECH'L OPENINGS.



TYPICAL FLOOR OR ROOF OPENING FRAME DETAIL

SECTION

SCALE: 1 1/2" = 1'-0 SRGS059A — C S0.4

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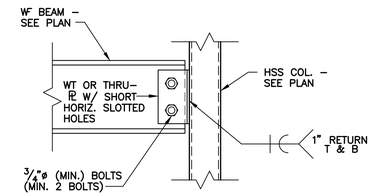
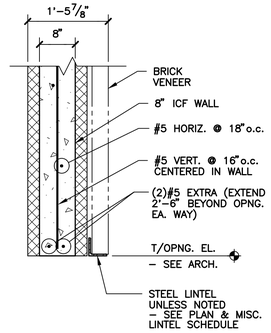


CONNECTIONS TO BE DESIGNED BY STRUCTURAL STEEL SUPPLIER IN ACCORDANCE WITH GENERAL NOTES

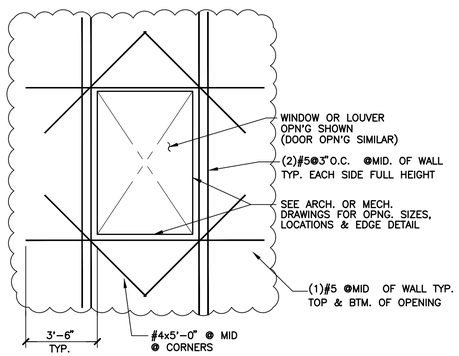
TYP. BEAM TO HSS COL. CONN.

SCALE: 1 1/2" = 1'-0 SOGS043A



TYP. WINDOW & DOOR HEAD DETAIL – ICF WALL

SCALE: 3/4" = 1'-0 CNES001F



TYPICAL @ OPENINGS IN CONCRETE WALLS

NOT TO SCALE CODE005

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STATE OF KENTUCKY — CARL J. SLESSER — No. 19894 — LICENSED PROFESSIONAL ENGINEER — 12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

TYPICAL DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

BG 15-229

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PHASE - 3

S0.4

HS

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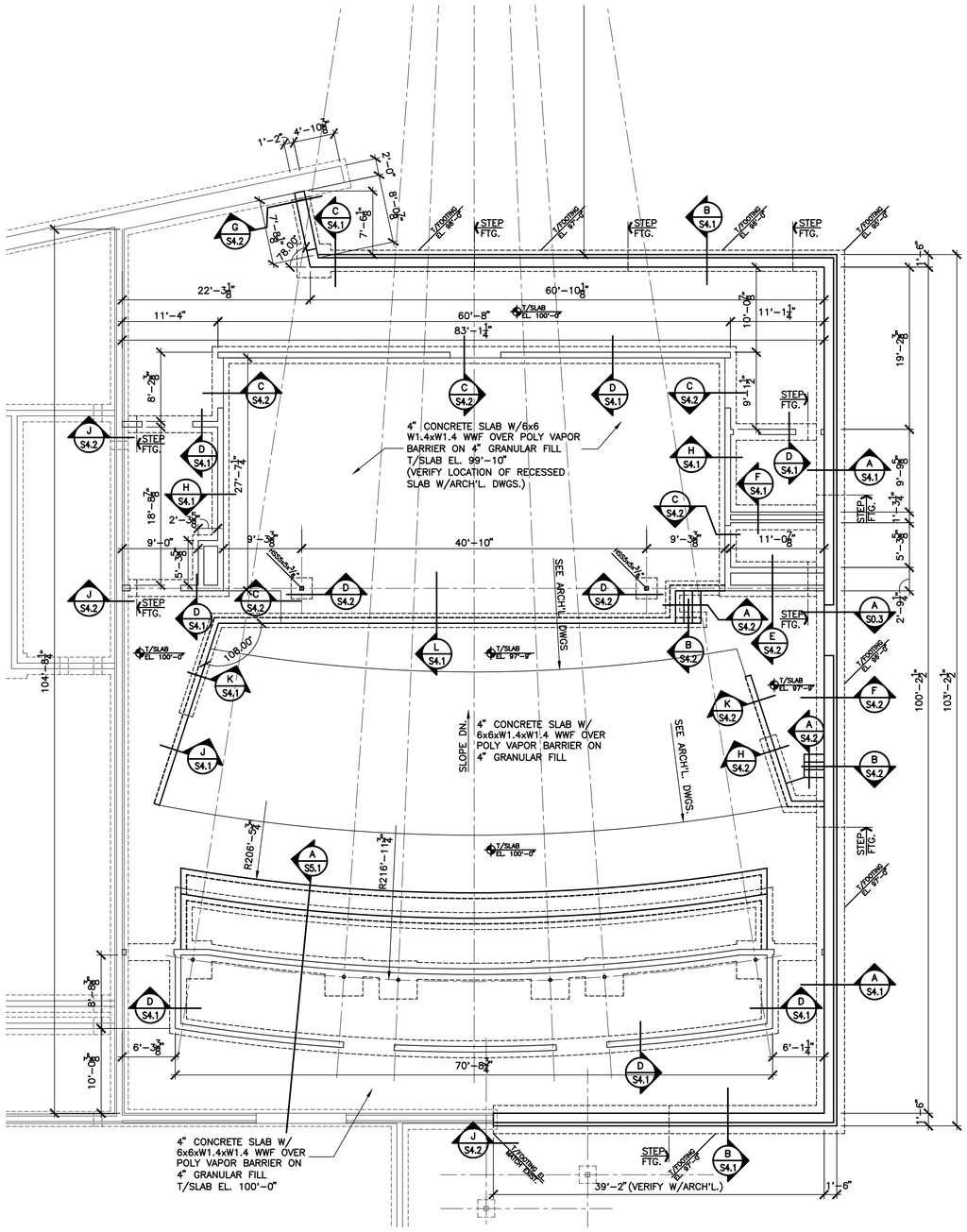

PLAN NORTH

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES:
1. SEE SHEETS S0.1, S0.2, S0.3 AND S0.4 FOR GENERAL NOTES AND TYPICAL DETAILS
2. USGS EL. 690.00 EL. 100'-0"
3. TOP OF EXTERIOR WALL FOOTING EL. 98'-0" UNLESS NOTED OTHERWISE.

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3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

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#10894
LICENSED PROFESSIONAL ENGINEER
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

FOUNDATION PLAN

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

DESIGNED TO EARN THE ENERGY STAR

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PHASE - 3

S1.1

HS

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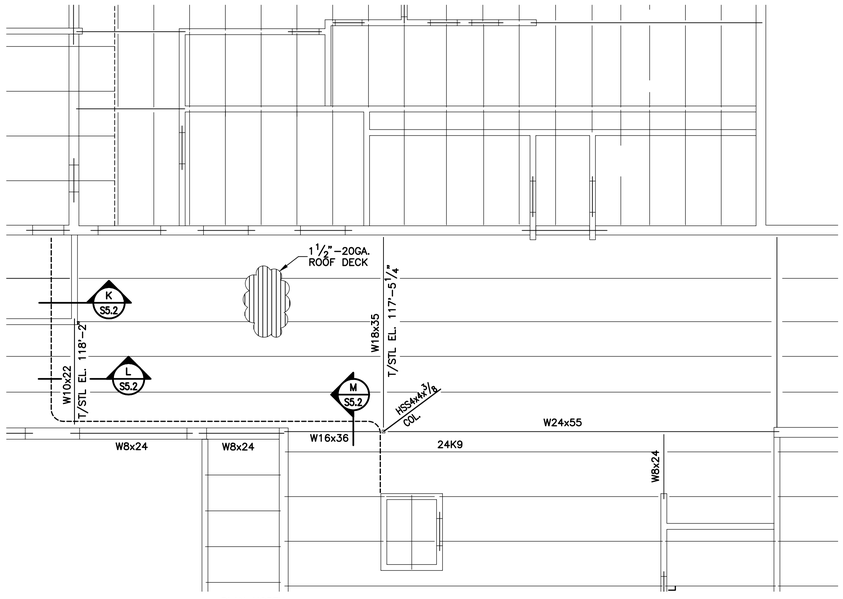
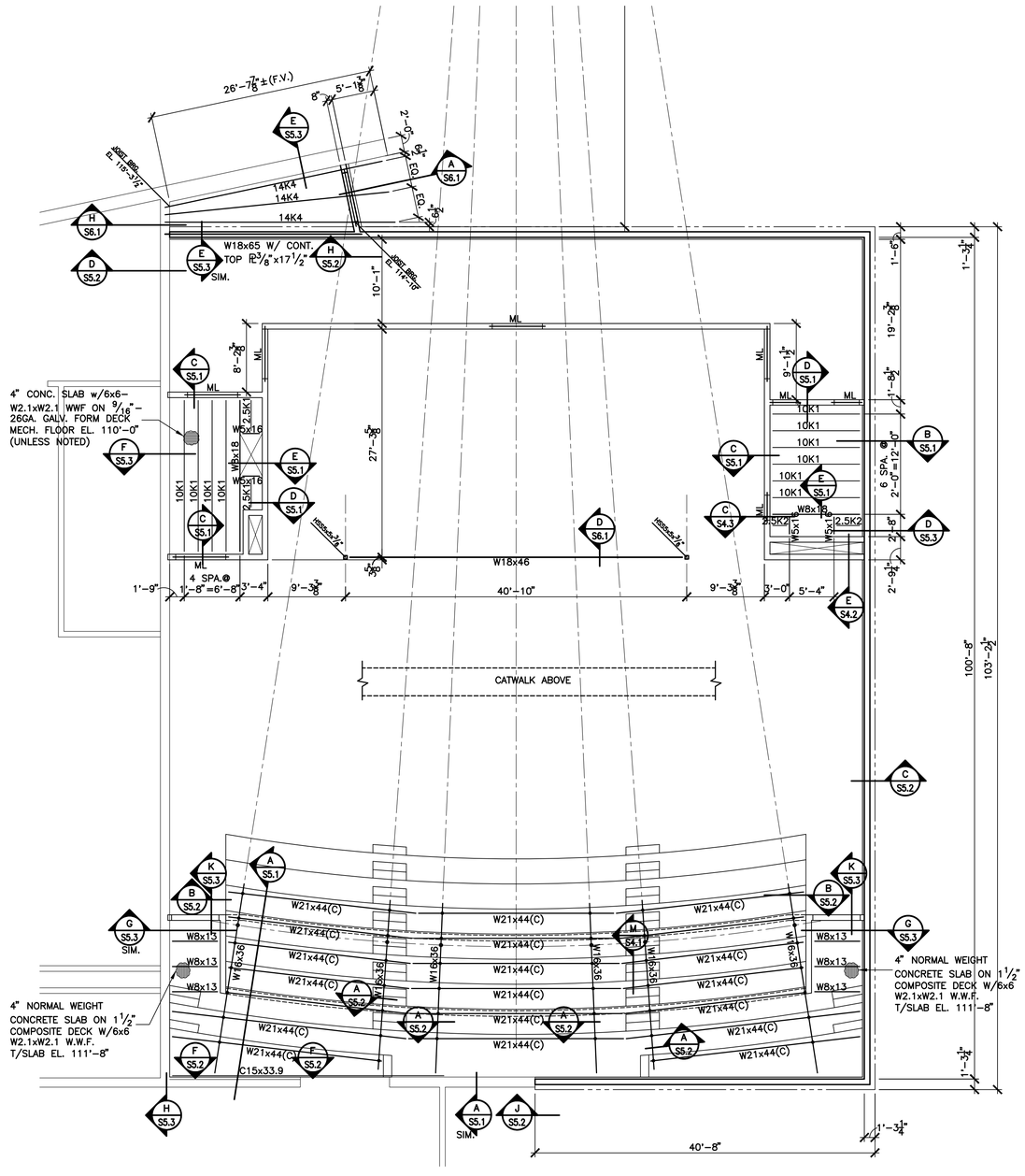


PLAN NORTH

SECURITY GRILLE PLAN

SCALE: 1/8" = 1'–0"



PLAN NORTH

UPPER FLOOR & MEZZANINE PLAN

SCALE: 1/8" = 1'–0"

NOTES:
1. SEE SHEETS S0.1, S0.2, S0.3 AND S0.4 FOR GENERAL NOTES AND TYPICAL DETAILS
2. W21x44(C) INDICATES COMPOSITE BEAM W⅝"øx3" HEADED STUDS AT 2'–0" O.C. SEE SECTION A/S5.1

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3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

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CARL J. SLESSER
#10994
LICENSED PROFESSIONAL ENGINEER
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

UPPER FLOOR & MEZZANINE PLAN

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

DESIGNED TO COMPLY WITH THE 2012 KENTUCKY BUILDING CODE

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PHASE - 3

S2.1

HS

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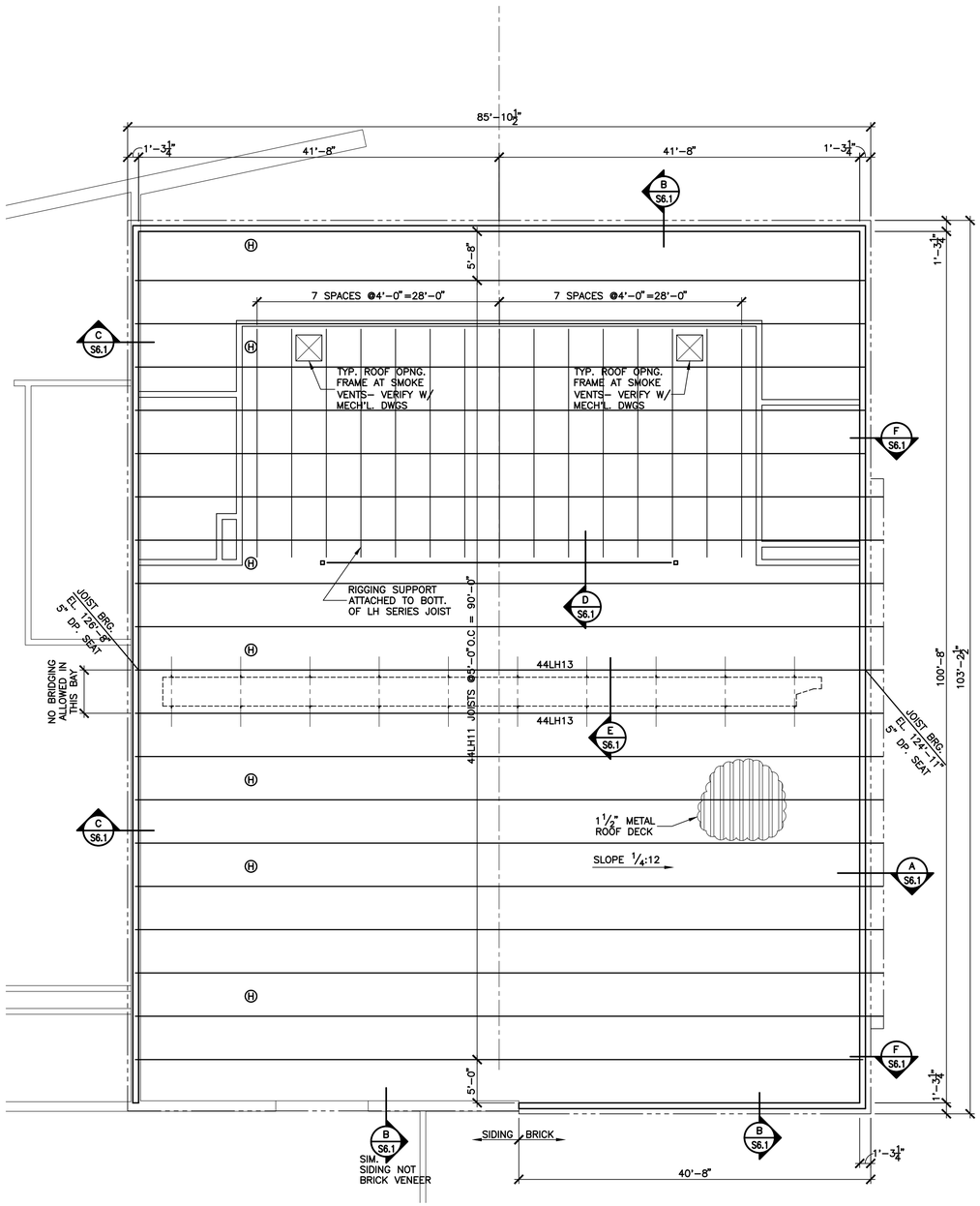

PLAN NORTH

ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

NOTES:
1. SEE SHEETS S0.1, S0.2, S0.3 AND S0.4 FOR GENERAL NOTES AND TYPICAL DETAILS
2. ONLY HORIZONTAL JOIST BRIDGING ALLOWED AT JOIST BAYS INDICATED THUS Ⓗ ON THE PLAN TO ALLOW FOR HVAC DUCTS.

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3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

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CARL J. SLESSER
#10894
LICENSED PROFESSIONAL ENGINEER
12/8/15

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288 WILDCAT LANE
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ROOF FRAMING PLAN

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
DESIGNED TO MEET ENERGY STAR
2885 NEW SHEPHERDSVILLE ROAD (HWY. 245) BARDSTOWN, KENTUCKY 40004

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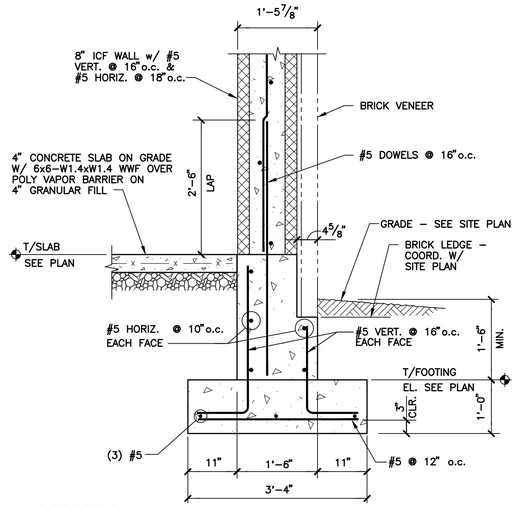
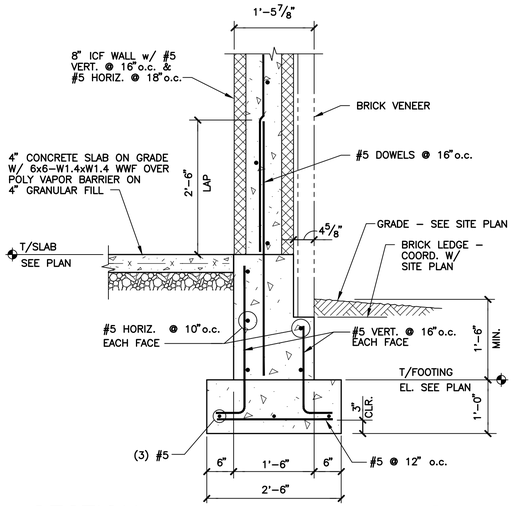
PHASE - 3

S3.1
HS



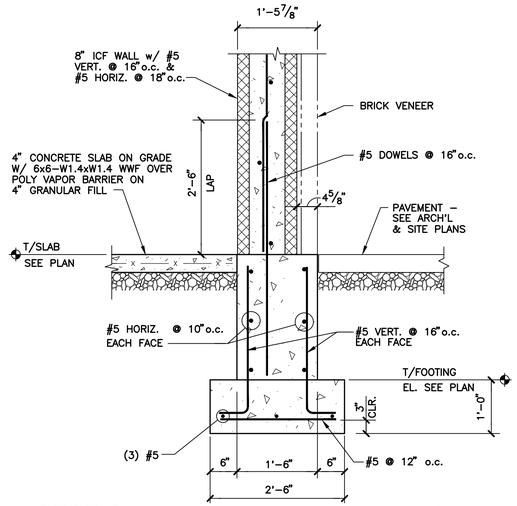
SECTION — CFES2309F — A — S4.1

SCALE: 3/4" = 1'-0



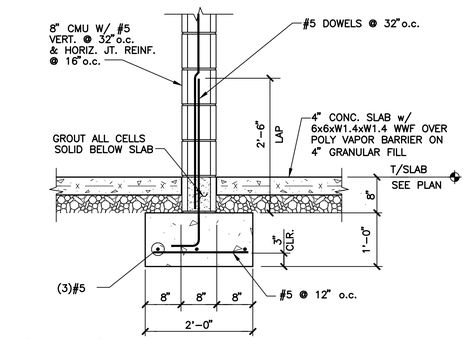
SECTION — CFES2309G — B — S4.1

SCALE: 3/4" = 1'-0



SECTION — CFES2309H — C — S4.1

SCALE: 3/4" = 1'-0



SECTION — CFES2309J — D — S4.1

SCALE: 3/4" = 1'-0

SECTION (TYPICAL THICKENED SLAB) — CFES2308M — E — S4.1

SCALE: 3/4" = 1'-0

SECTION (TYPICAL CHASE WALL UNLESS NOTED) — CFES2167E — F — S4.1

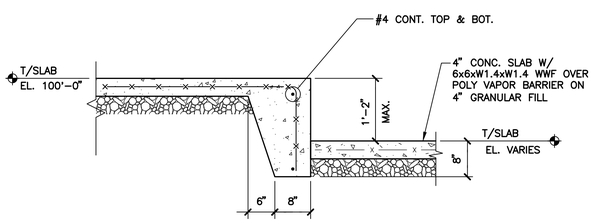
SCALE: 3/4" = 1'-0

TYP. WALL/SLAB EDGE — CFES2262H — G — S4.1

SCALE: 3/4" = 1'-0

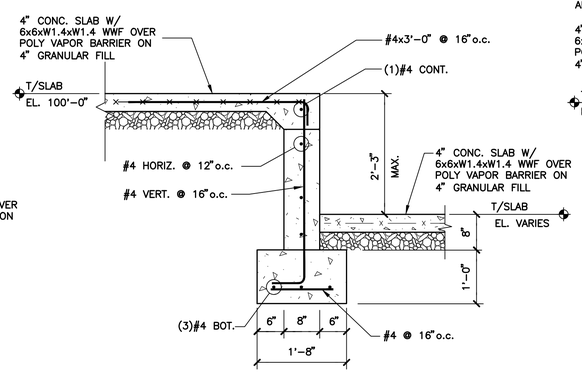
SECTION — CFES2374C — H — S4.1

SCALE: 3/4" = 1'-0



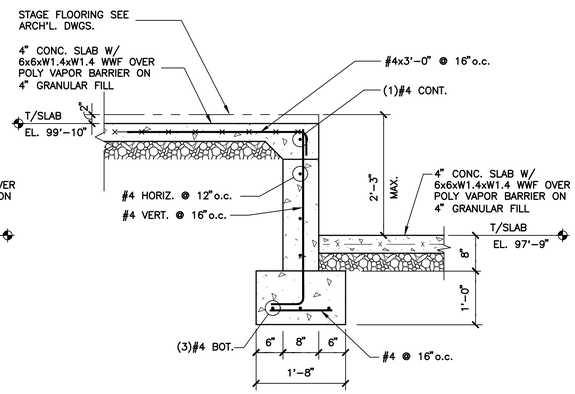
SECTION — CFES2374 — J — S4.1

SCALE: 3/4" = 1'-0



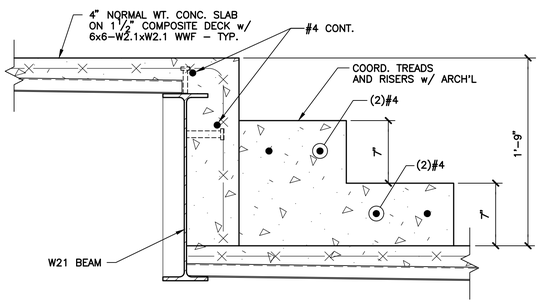
SECTION — CFES2374A — K — S4.1

SCALE: 3/4" = 1'-0



SECTION — CFES2374B — L — S4.1

SCALE: 3/4" = 1'-0



SECTION (TYP. STAIR @ STADIUM SEATING) — SLGS222D — M — S4.1

SCALE: 1 1/2" = 1'-0

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STRUCTURAL ENGINEERS
2325 LIME KILN LANE
LOUISVILLE, KENTUCKY 40222

studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

STATE OF KENTUCKY
CARL J. SLESSER
#10994
LICENSED PROFESSIONAL ENGINEER
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

SECTIONS & DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
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S4.1

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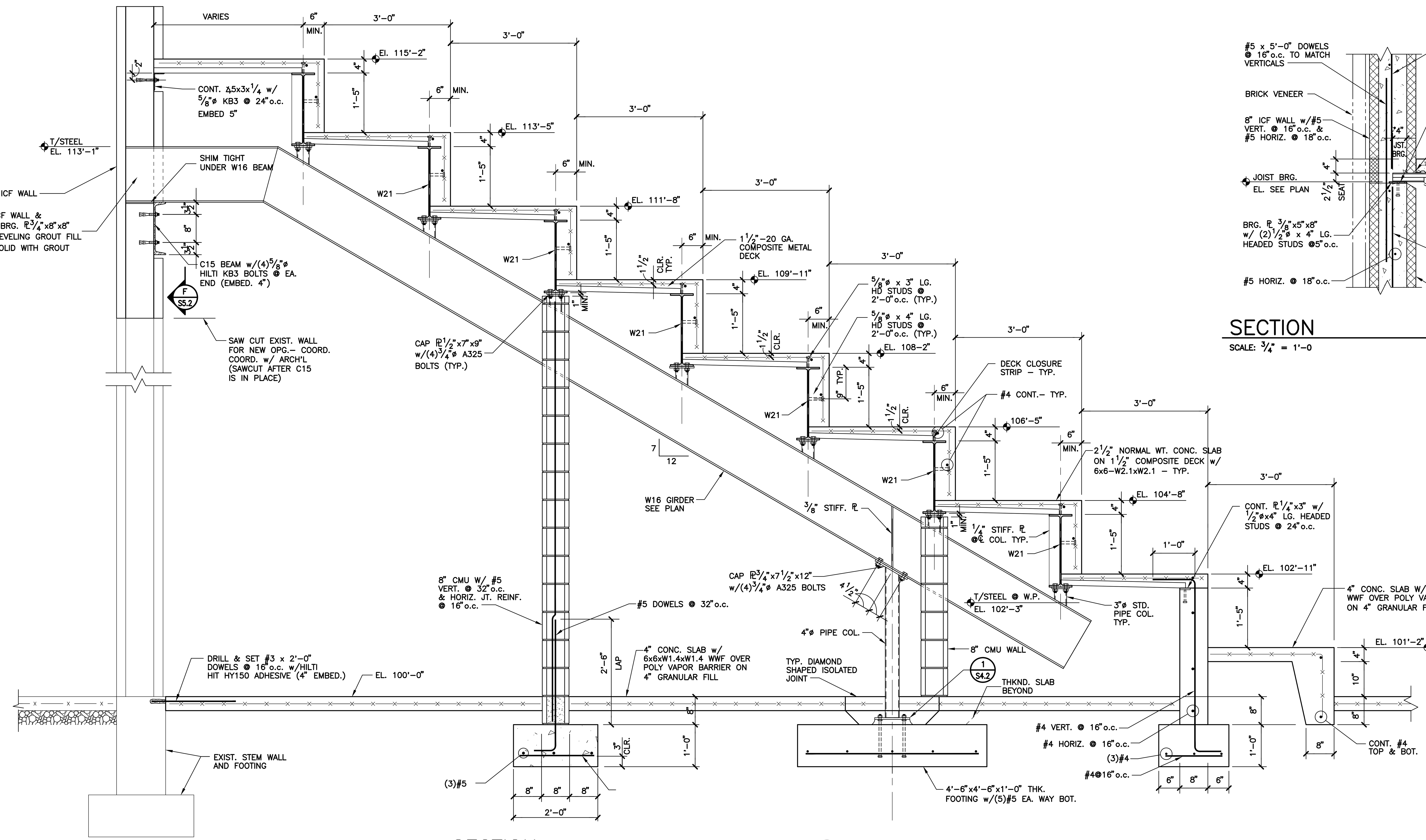
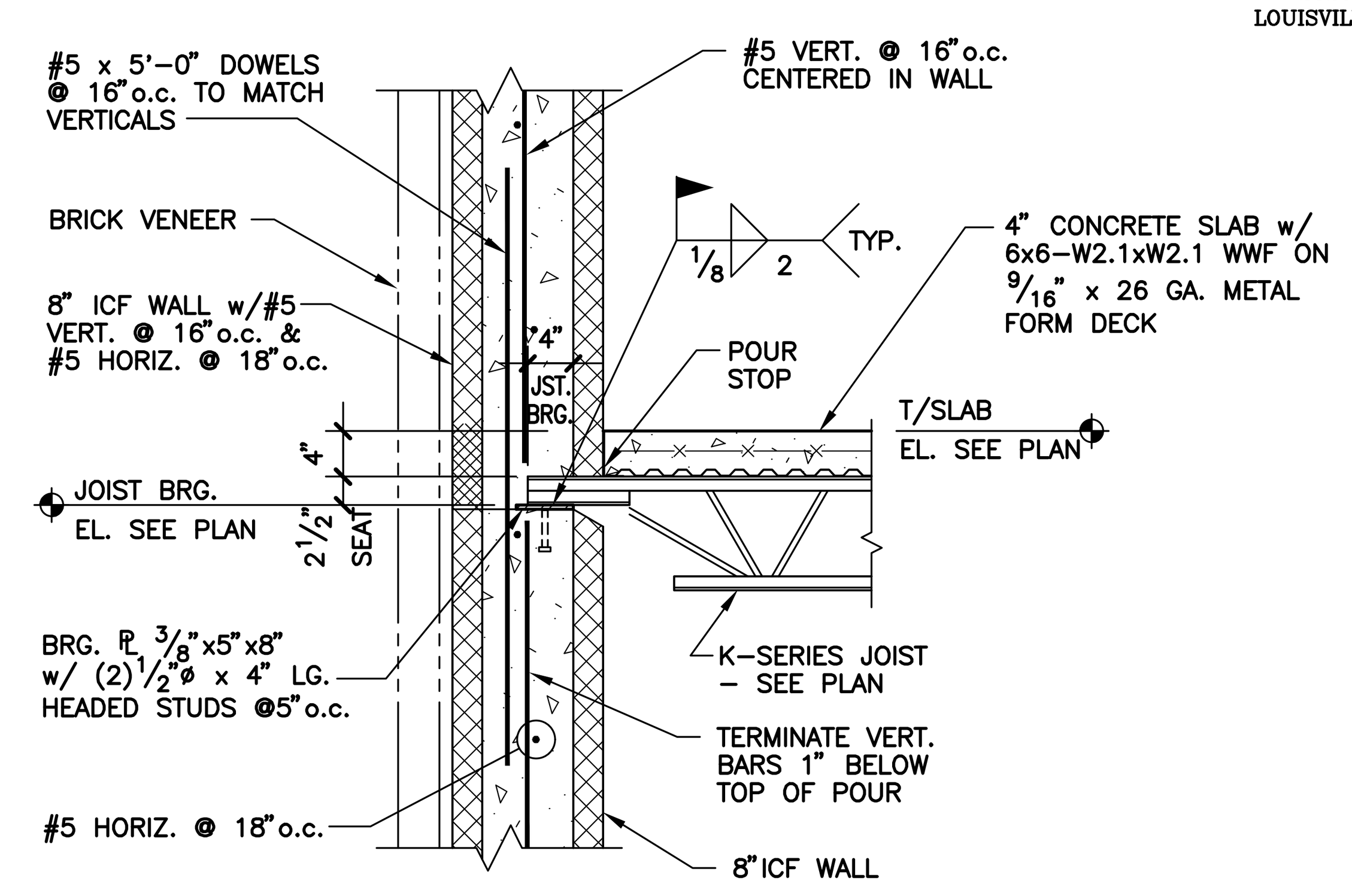
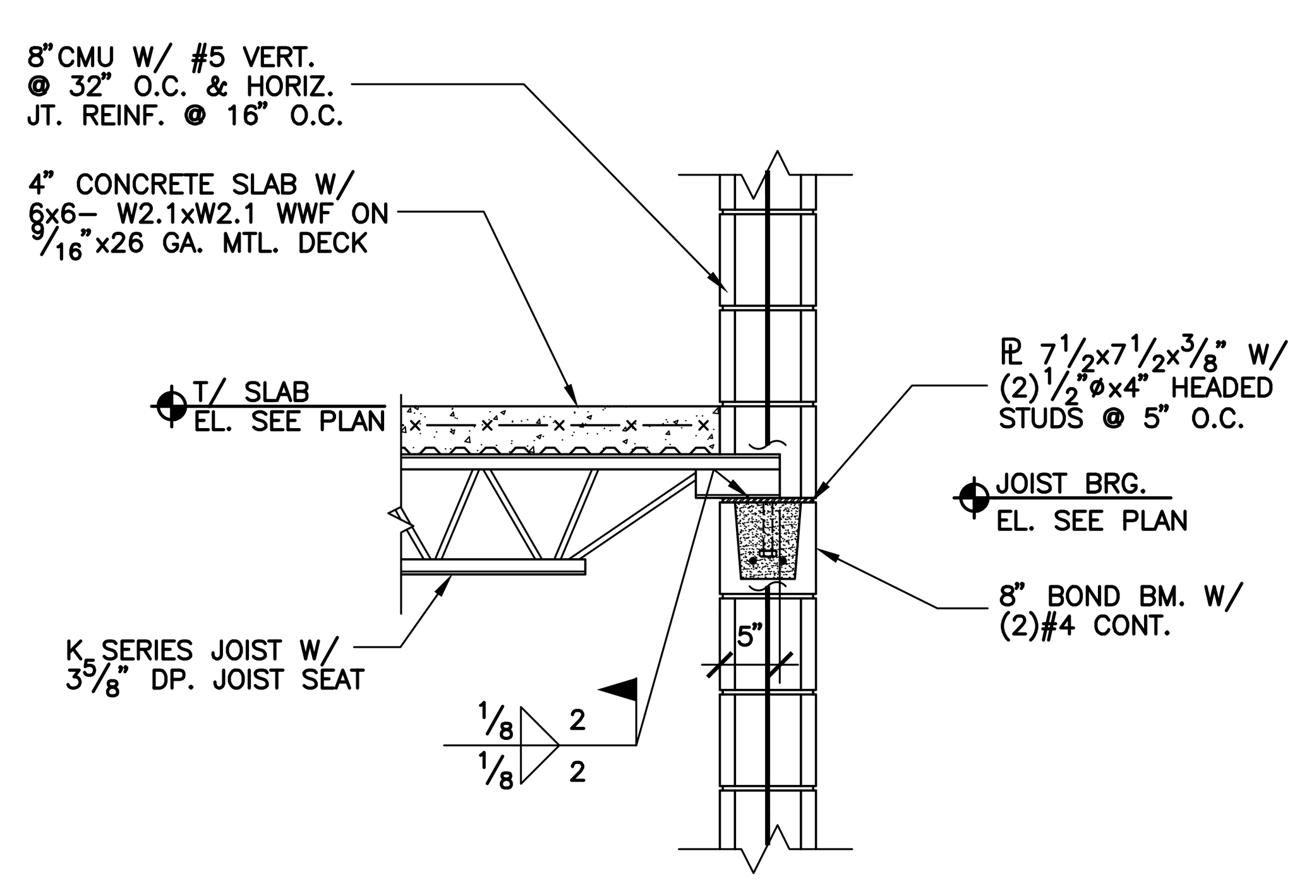


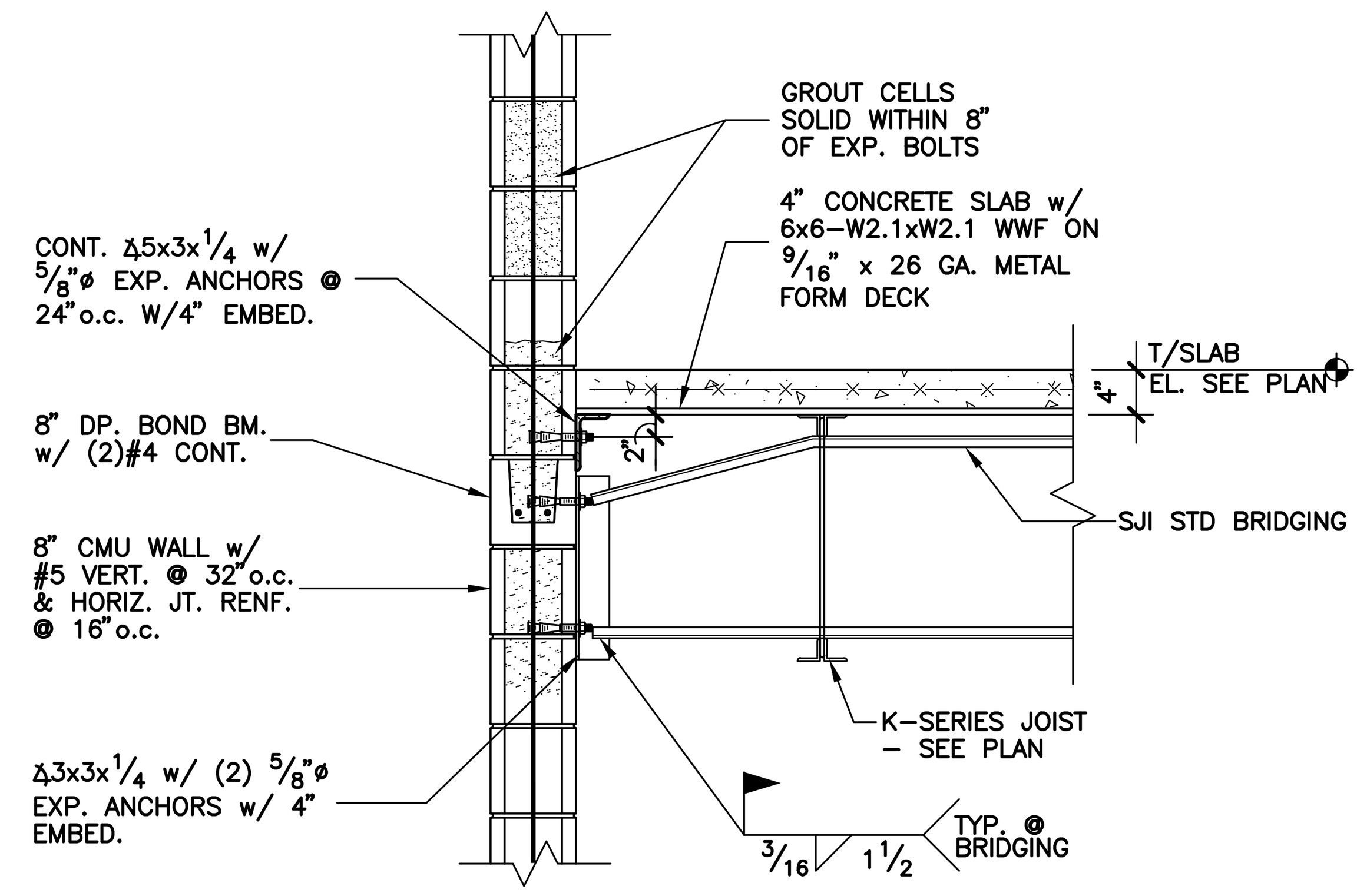
SECTION A/S5.1
SCALE: 3/4" = 1'-0
SLES321D



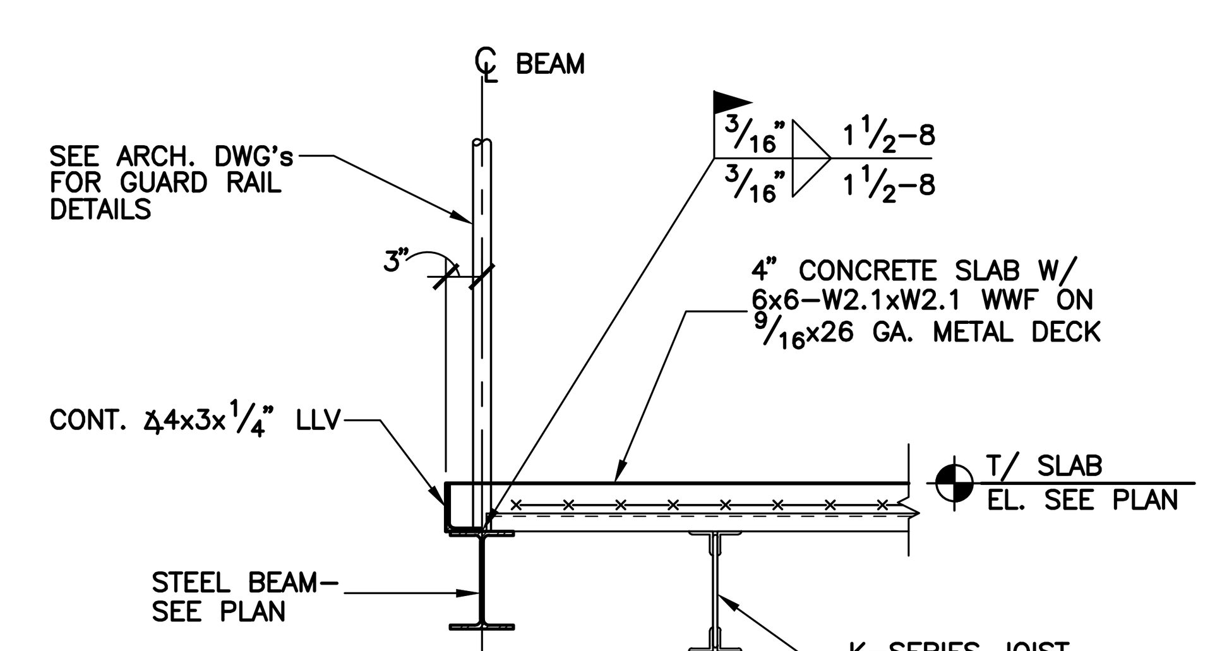
SECTION B/S5.1
SCALE: 3/4" = 1'-0
MLES122H



SECTION C/S5.1
SCALE: 3/4" = 1'-0
MLES131



SECTION D/S5.1
SCALE: 3/4" = 1'-0
MLES131A



SECTION E/S5.1
SCALE: 3/4" = 1'-0
MLES131B

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NELSON COUNTY SCHOOL DISTRICT
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SECTIONS & DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
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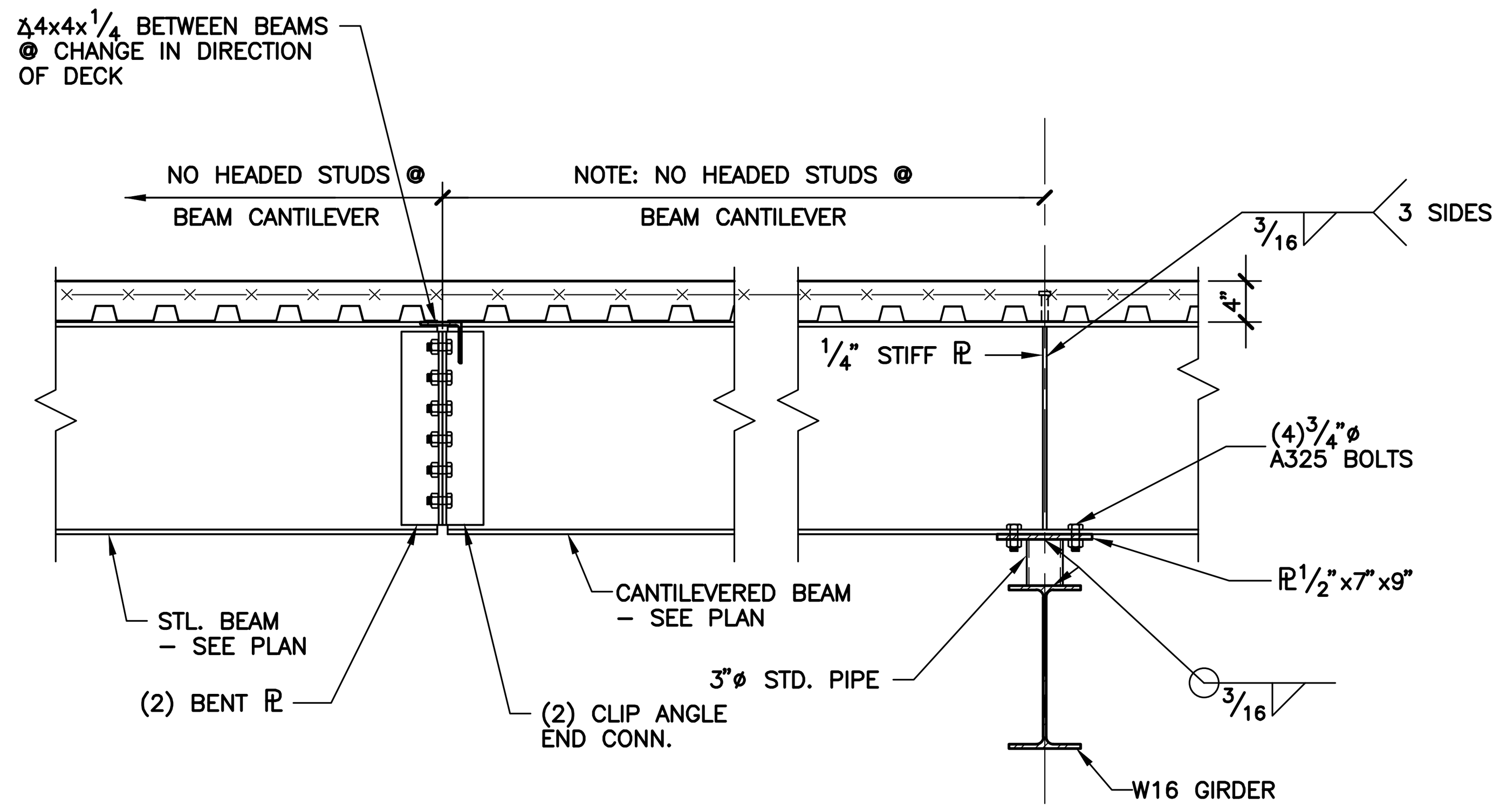
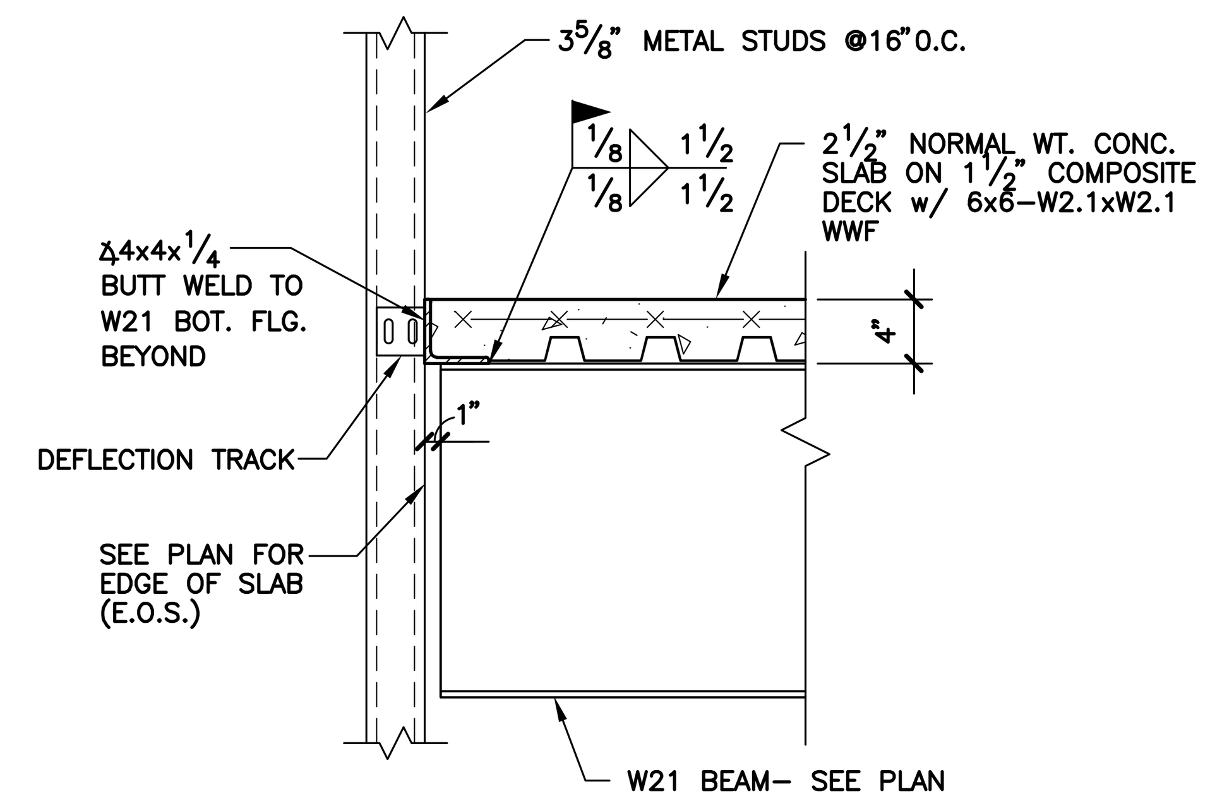
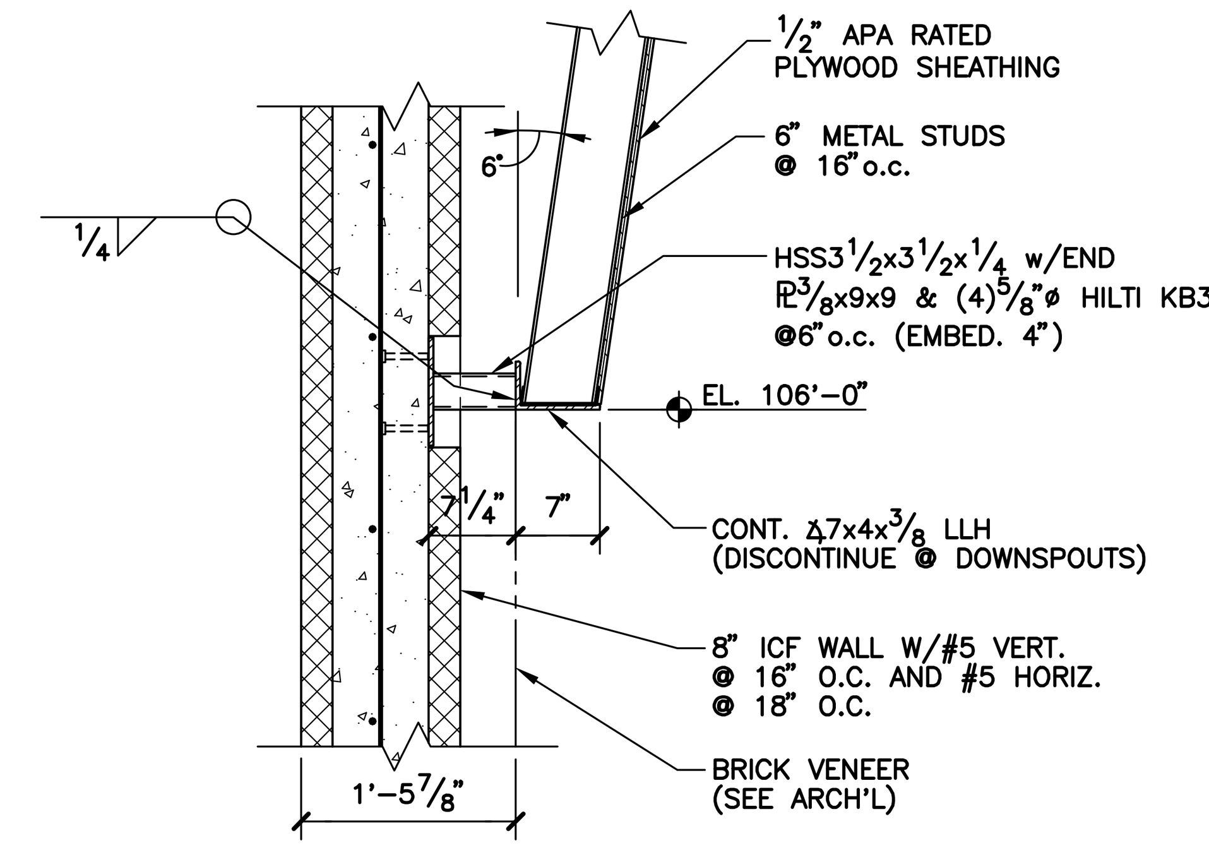


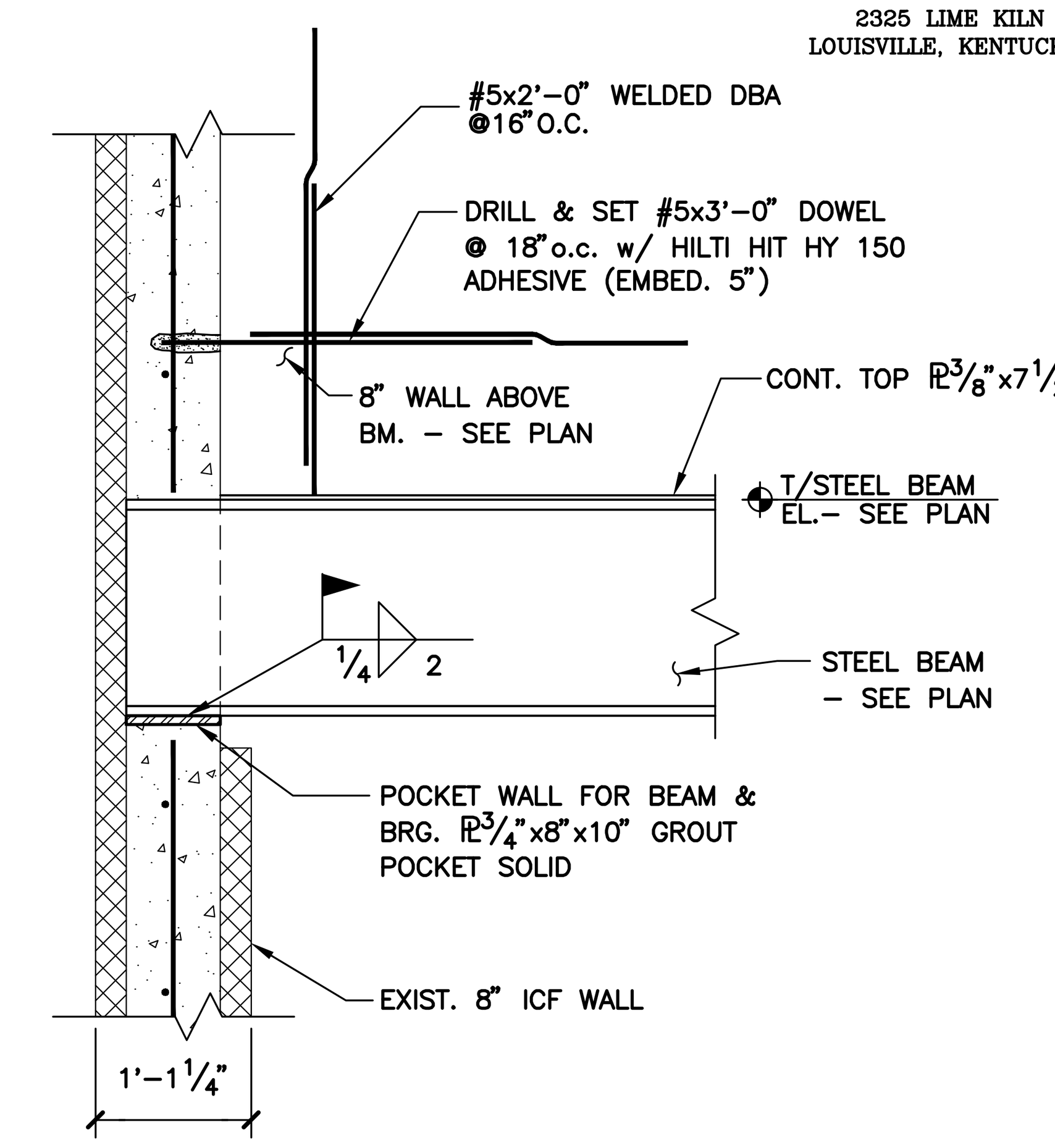
SECTION A / S5.2 SLES322A
SCALE: 3/4" = 1'-0



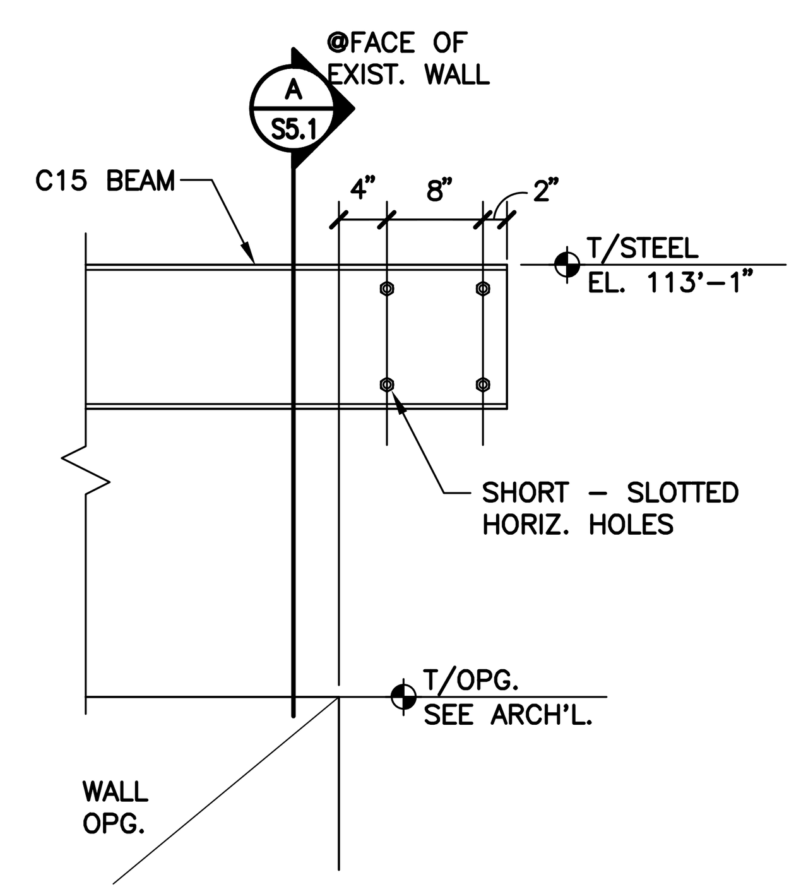
SECTION B / S5.2 SLFS291B
SCALE: 1" = 1'-0



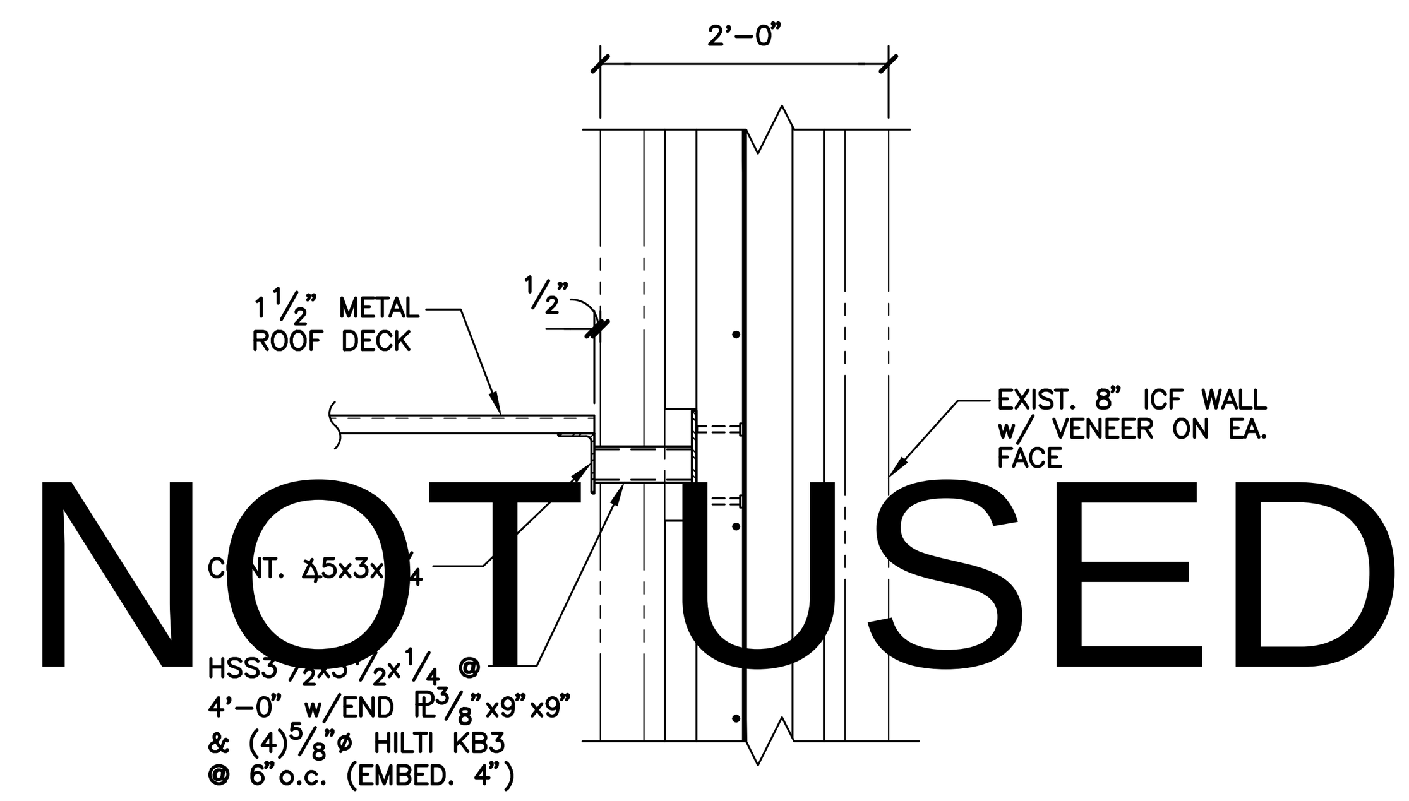
SECTION C / S5.2 COES096
SCALE: 3/4" = 1'-0



SECTION D / S5.2 COES096A
SCALE: 3/4" = 1'-0



SECTION F / S5.2 COES096C
SCALE: 3/4" = 1'-0



SECTION G / S5.2 COES096D
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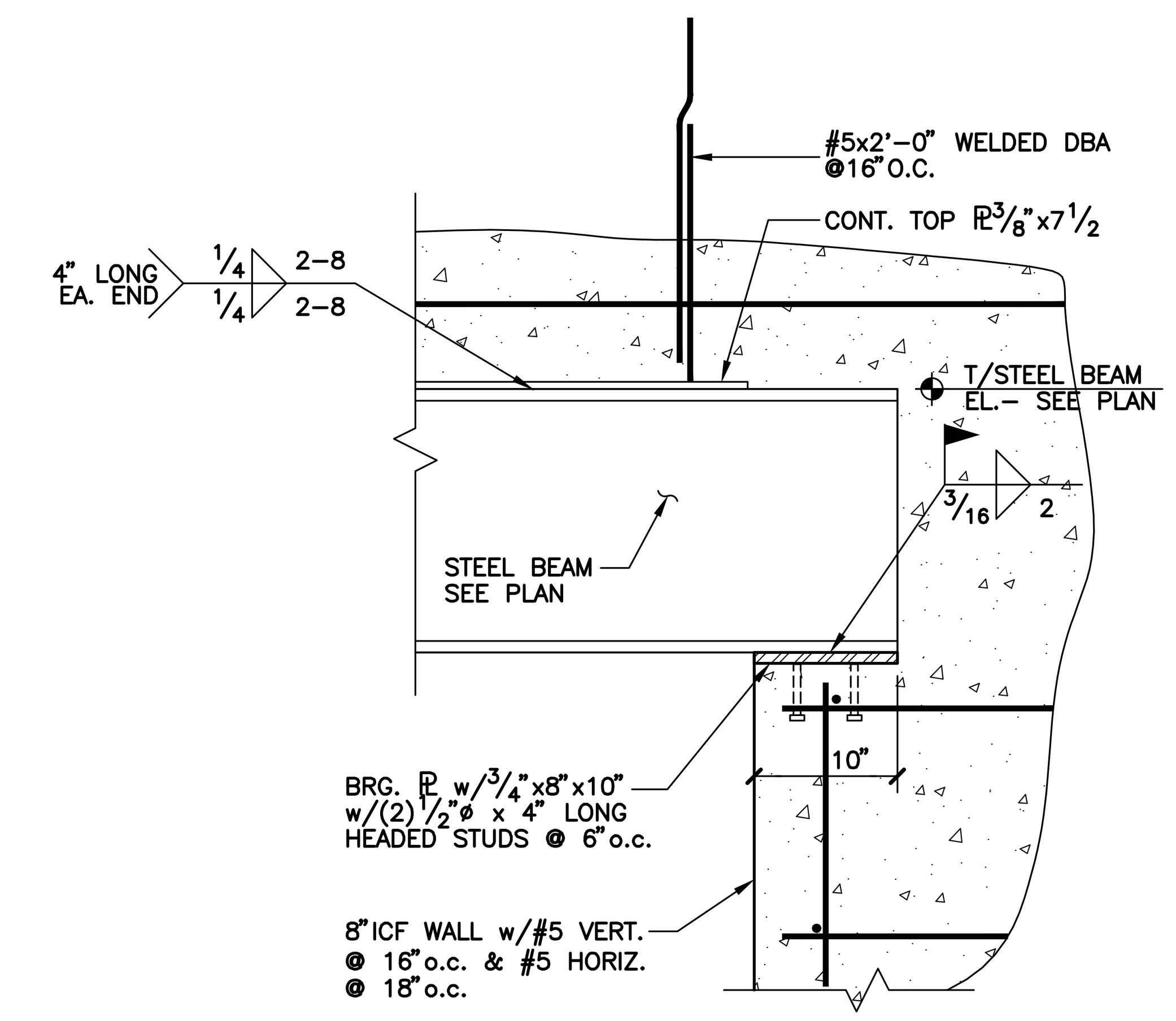

SECTION H / S5.2 CFFS1057
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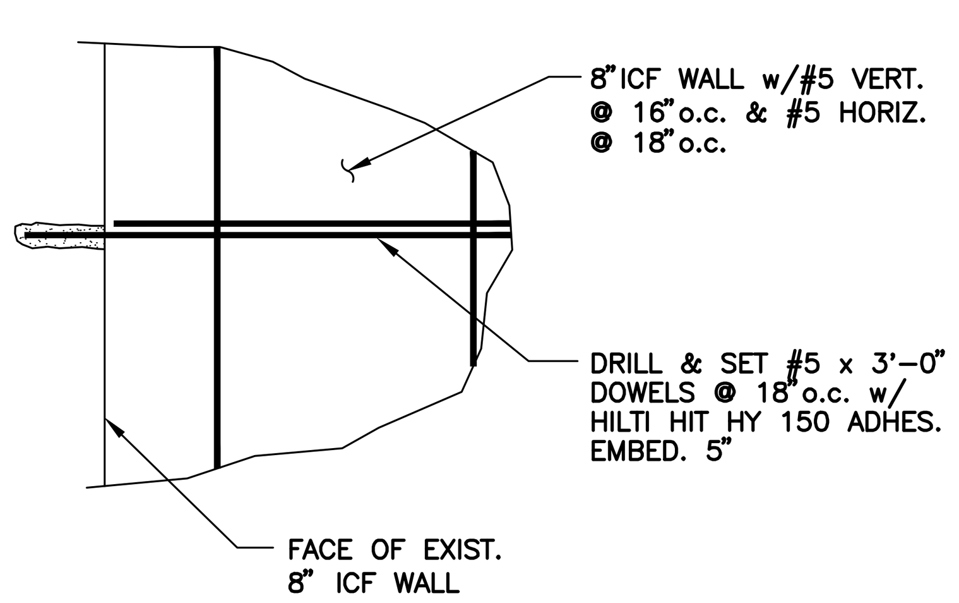
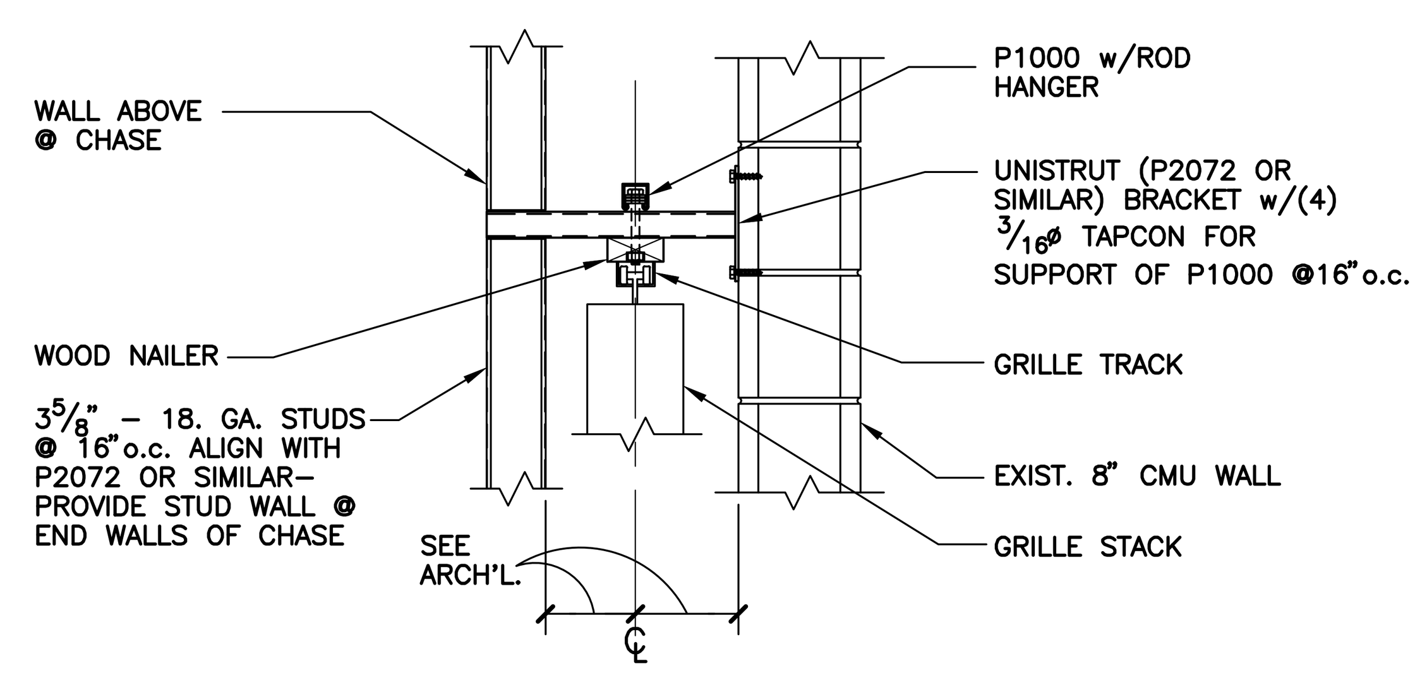


SECTION J / S5.2 CFFS1057A
SCALE: 1" = 1'-0



SECTION K / S5.2 CFFS1058
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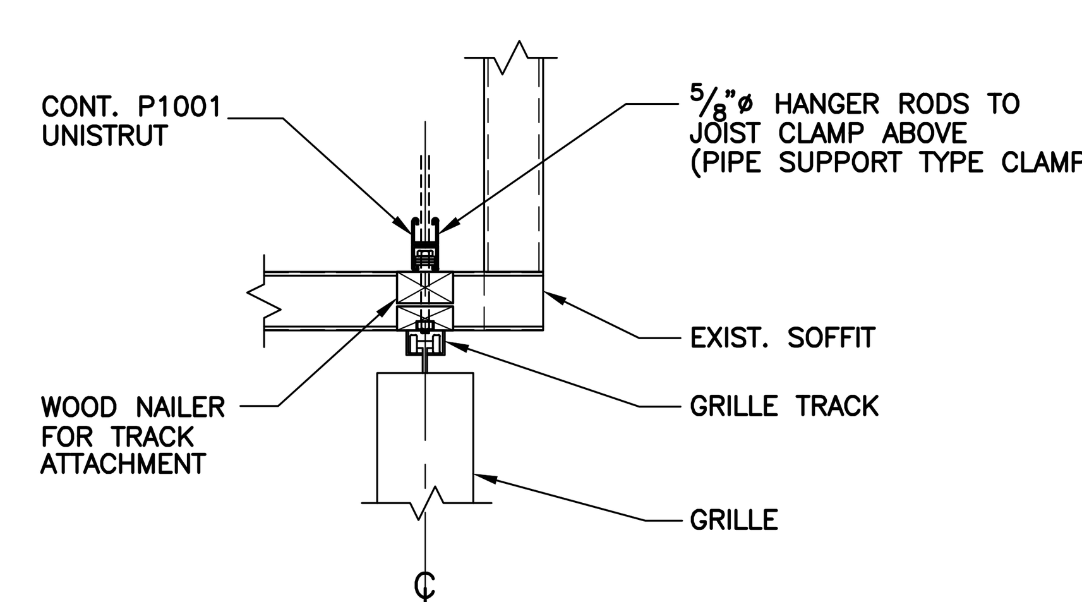

SECTION L / S5.2 CFFS1058A
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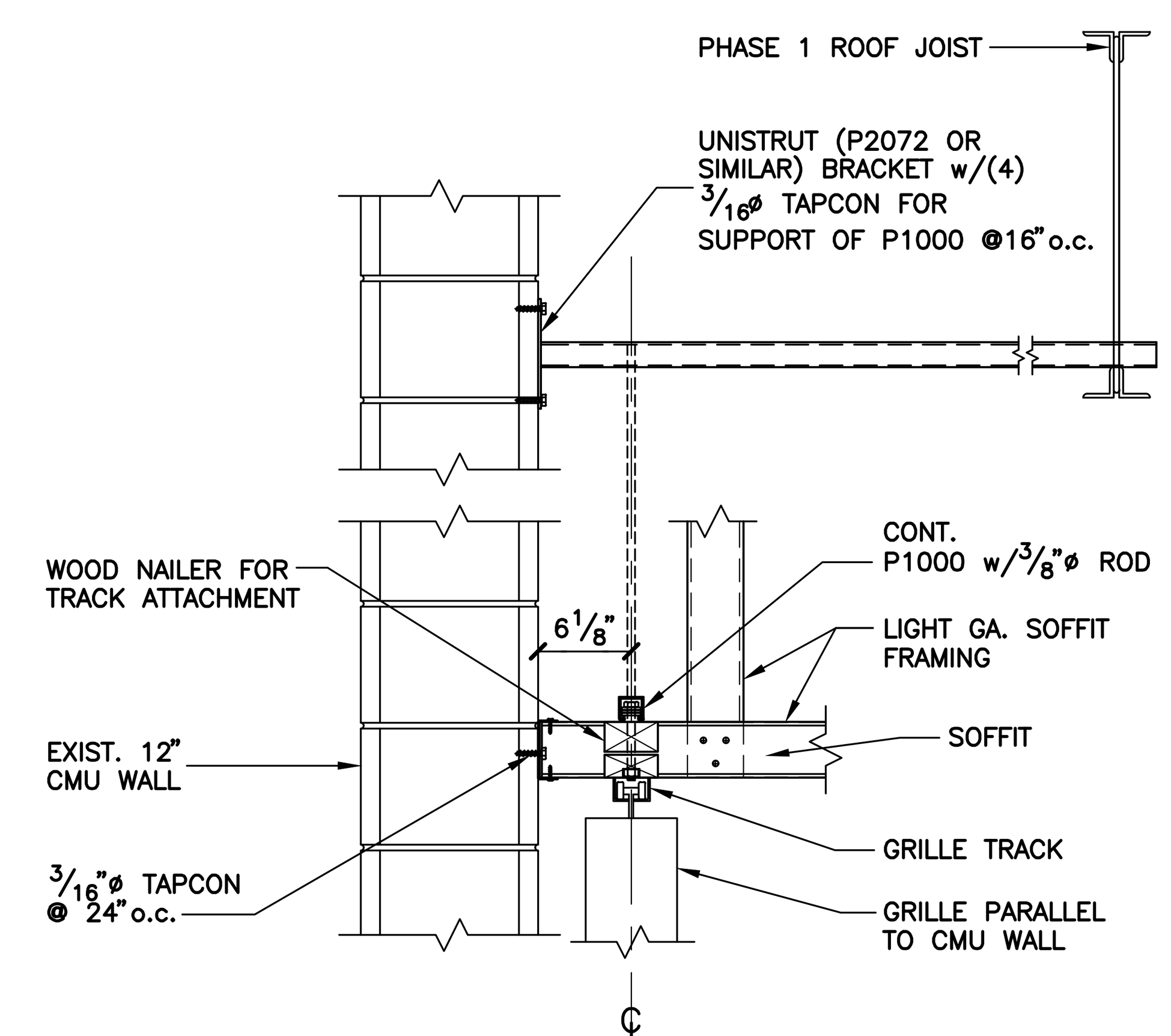

SECTION M / S5.2 CFFS1058B
SCALE: 1" = 1'-0

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STRUCTURAL ENGINEERS
2325 LIME KILN LANE
LOUISVILLE, KENTUCKY 40222

studio
studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

STATE OF KENTUCKY
CARL J. SLESSER
#10694
LICENSED PROFESSIONAL ENGINEER
[illegible]
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
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SECTIONS & DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
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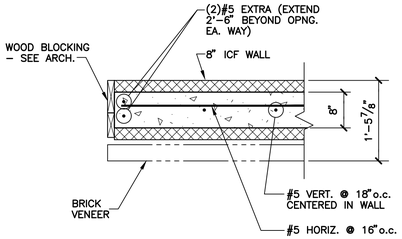
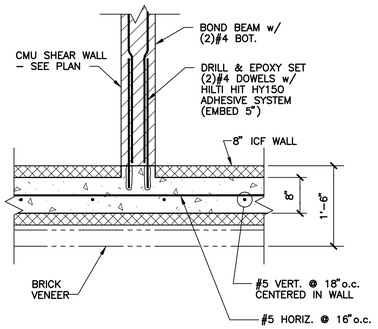


TYP. WINDOW OR DOOR JAMB DETAIL – ICF WALL

CNES001G — A / S5.3

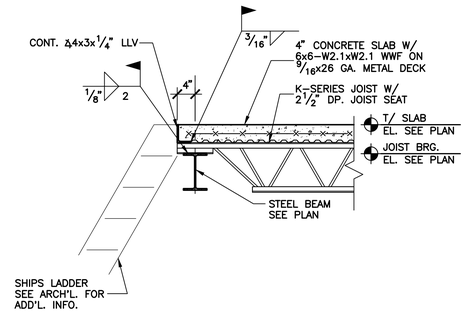
SCALE: 3/4" = 1'–0



TYP. CMU SHEAR WALL BOND BM. – ICF WALL INTERSECTION

CNES001H — B / S5.3

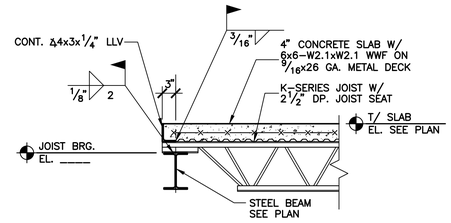
SCALE: 3/4" = 1'–0



SECTION

MLES114D — C / S5.3

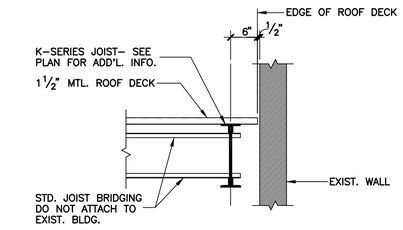
SCALE: 3/4" = 1'–0



SECTION

MLES114F — D / S5.3

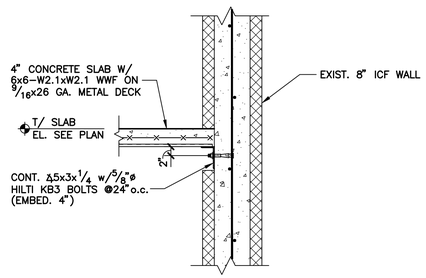
SCALE: 3/4" = 1'–0



SECTION

SRES1046 — E / S5.3

SCALE: 3/4" = 1'–0



SECTION

SRES1046A — F / S5.3

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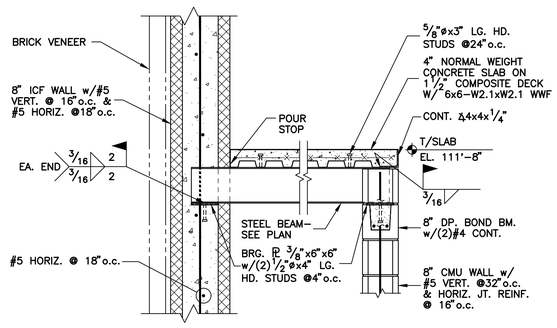
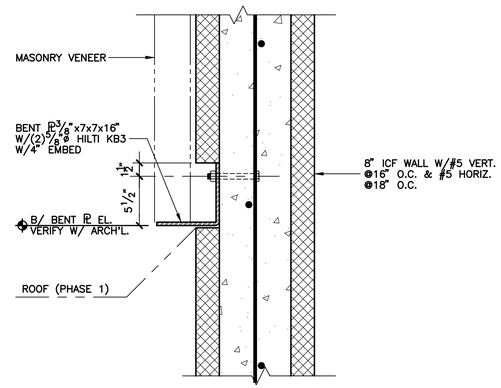


SECTION

MLES122J — G / S5.3

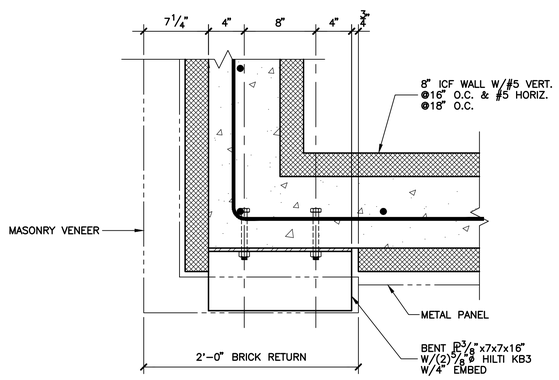
SCALE: 3/4" = 1'–0



SECTION

COGS016 — H / S5.3

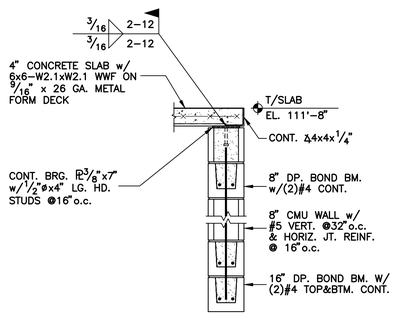
SCALE: 1 1/2" = 1'–0



DETAIL

COGD024 — J / S5.3

SCALE: 1 1/2" = 1'–0



SECTION

MLES122K — K / S5.3

SCALE: 3/4" = 1'–0

studio
studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

STATE OF KENTUCKY
CARL J. SLESSER
#10894
LICENSED PROFESSIONAL ENGINEER
12/8/15

NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
BARDSTOWN, KENTUCKY 40004

SECTIONS & DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

DESIGNED TO EARN THE ENERGY STAR

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S5.3

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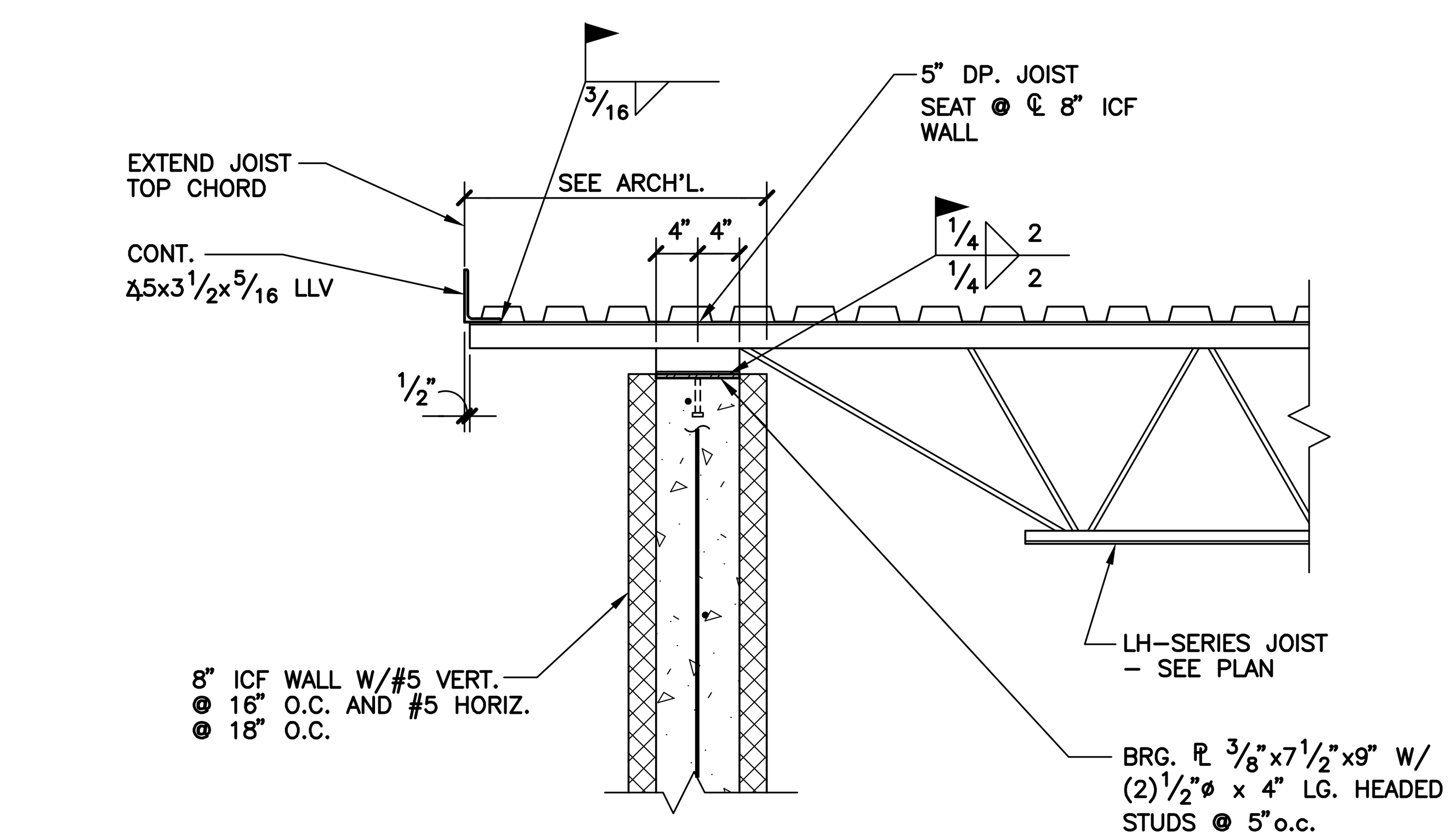
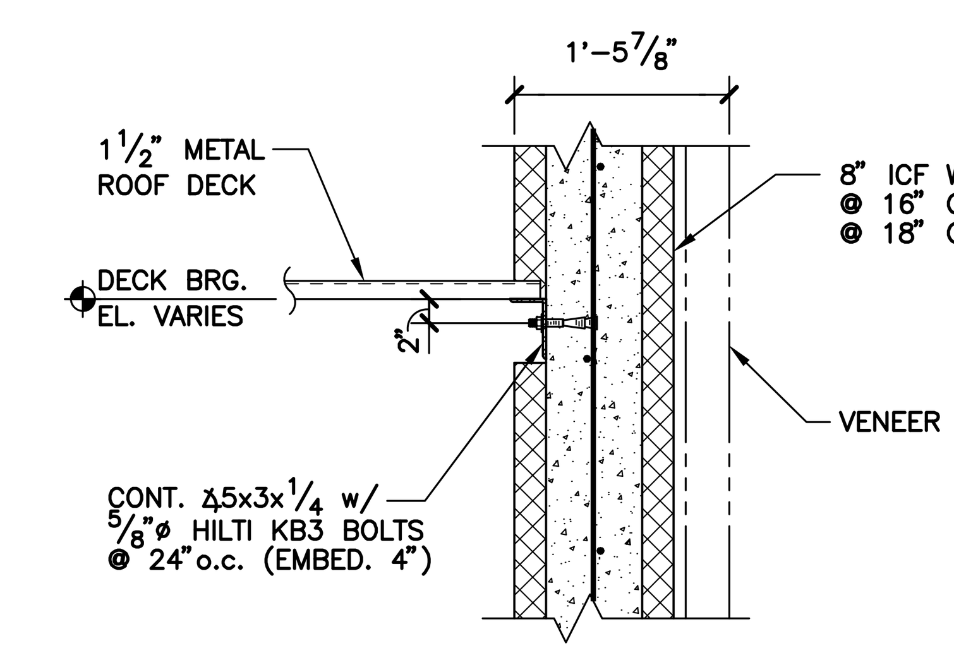


SECTION (A) S6.1

SCALE: 3/4" = 1'-0

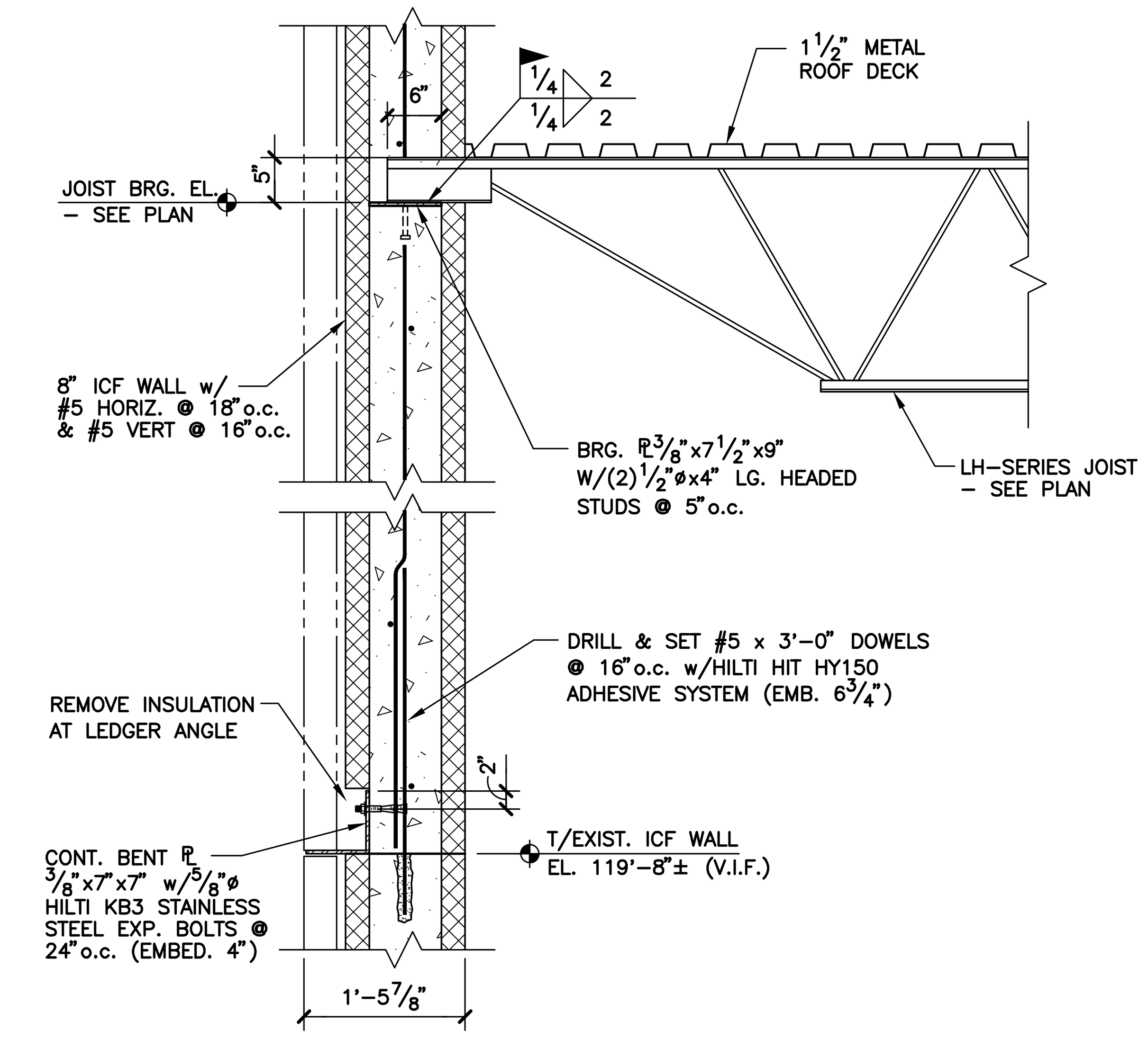
CLES231D



SECTION (B) S6.1

SCALE: 3/4" = 1'-0

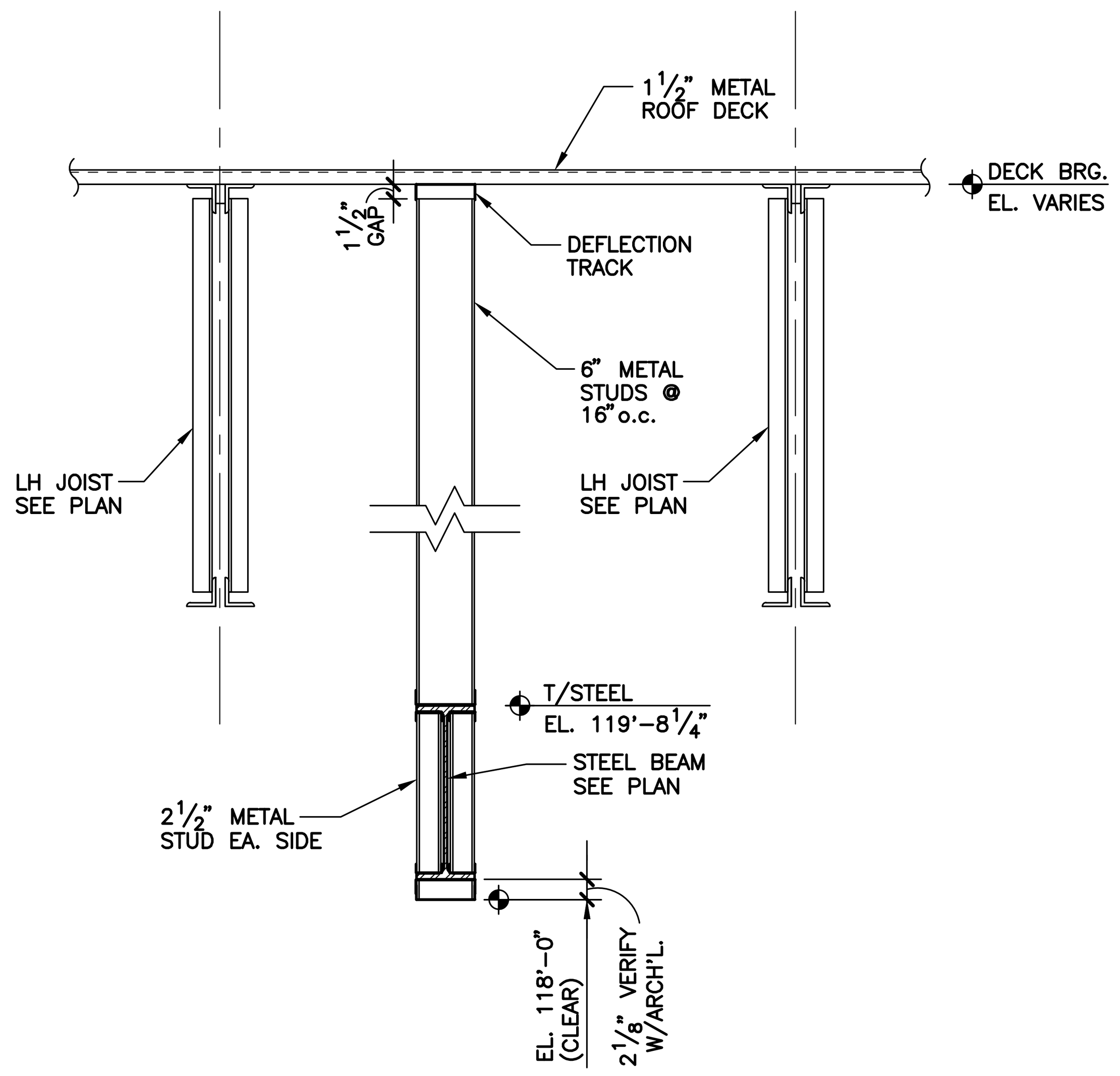
CLES231E



SECTION (C) S6.1

SCALE: 3/4" = 1'-0

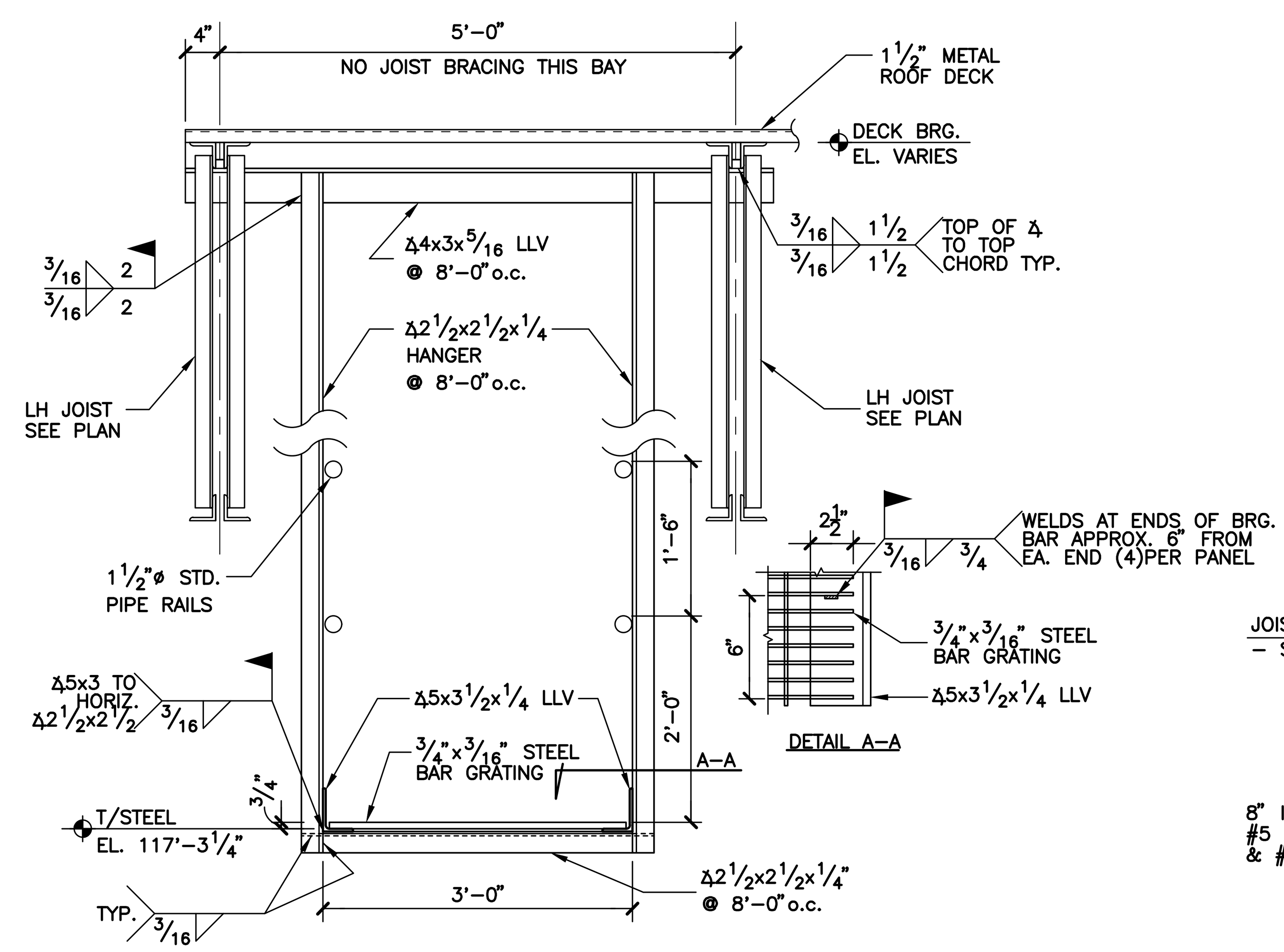
SRES1019F



SECTION (D) S6.1

SCALE: 3/4" = 1'-0

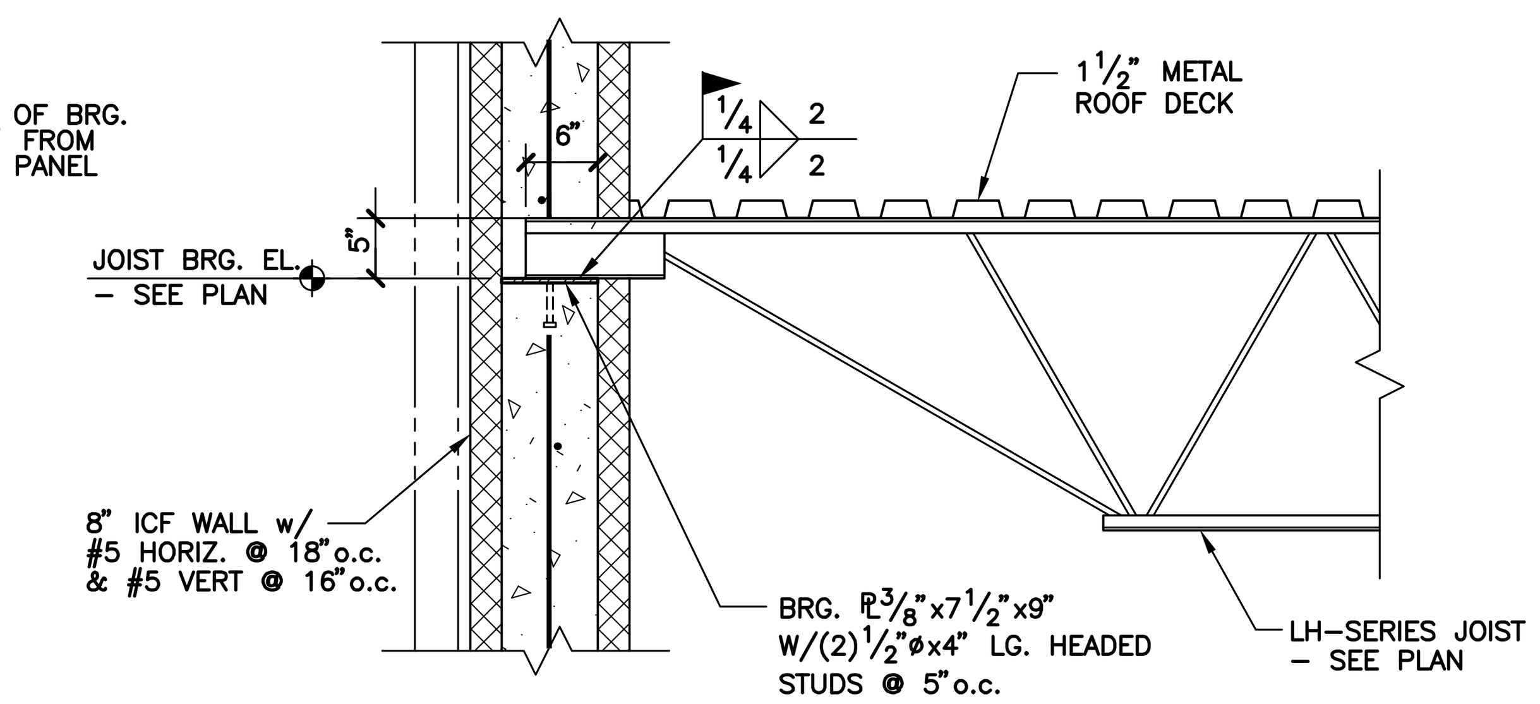
SRES1040A



SECTION (E) S6.1

SCALE: 3/4" = 1'-0

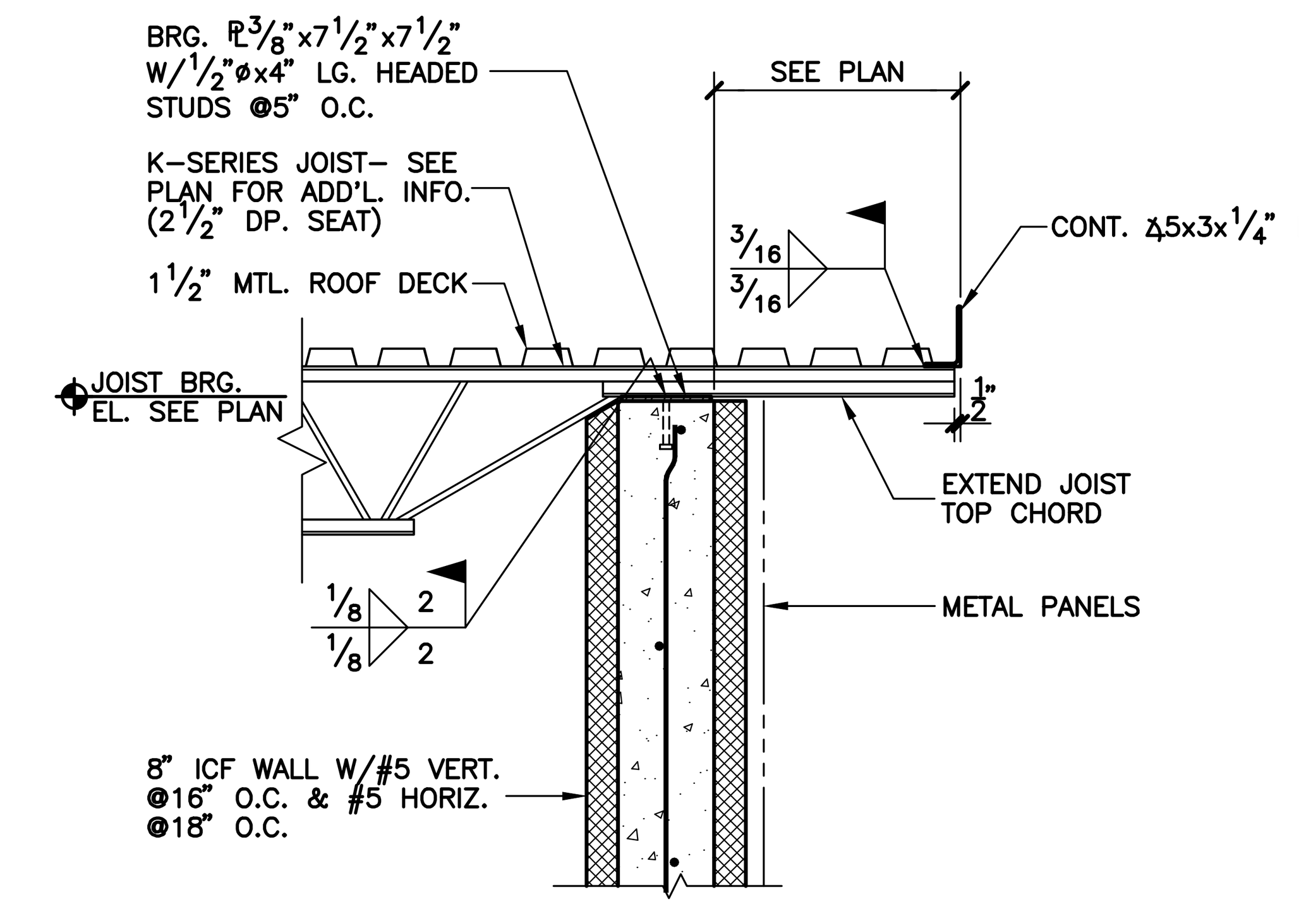
SRES1040



SECTION (F) S6.1

SCALE: 3/4" = 1'-0

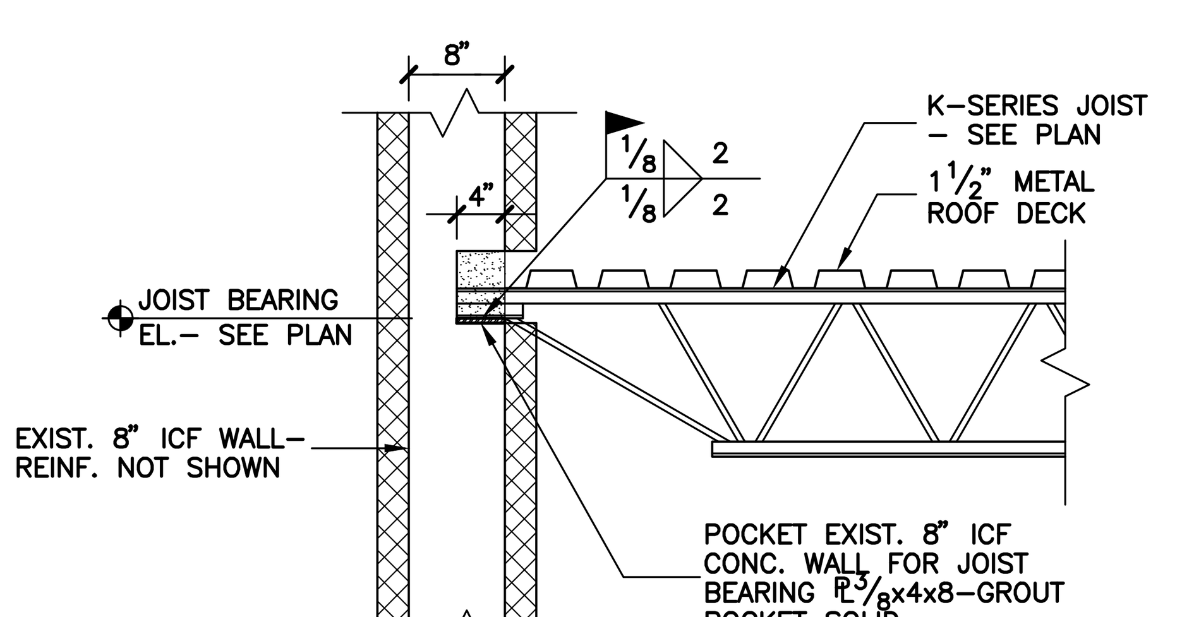
SRES1019F



SECTION (G) S6.1

SCALE: 3/4" = 1'-0

SRES1046B



SECTION (H) S6.1

SCALE: 3/4" = 1'-0

SRES1019G

SLESSER ENGINEERING, INC.
STRUCTURAL ENGINEERS
2325 LIME KILN LANE
LOUISVILLE, KENTUCKY 40222

studio
studio kremer architects
3258 Ruckriegel Parkway, Louisville, KY 40299
TEL 502.499.1100 FAX 499.1101

STATE OF KENTUCKY
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#10694
LICENSED PROFESSIONAL ENGINEER
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NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
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SECTIONS & DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
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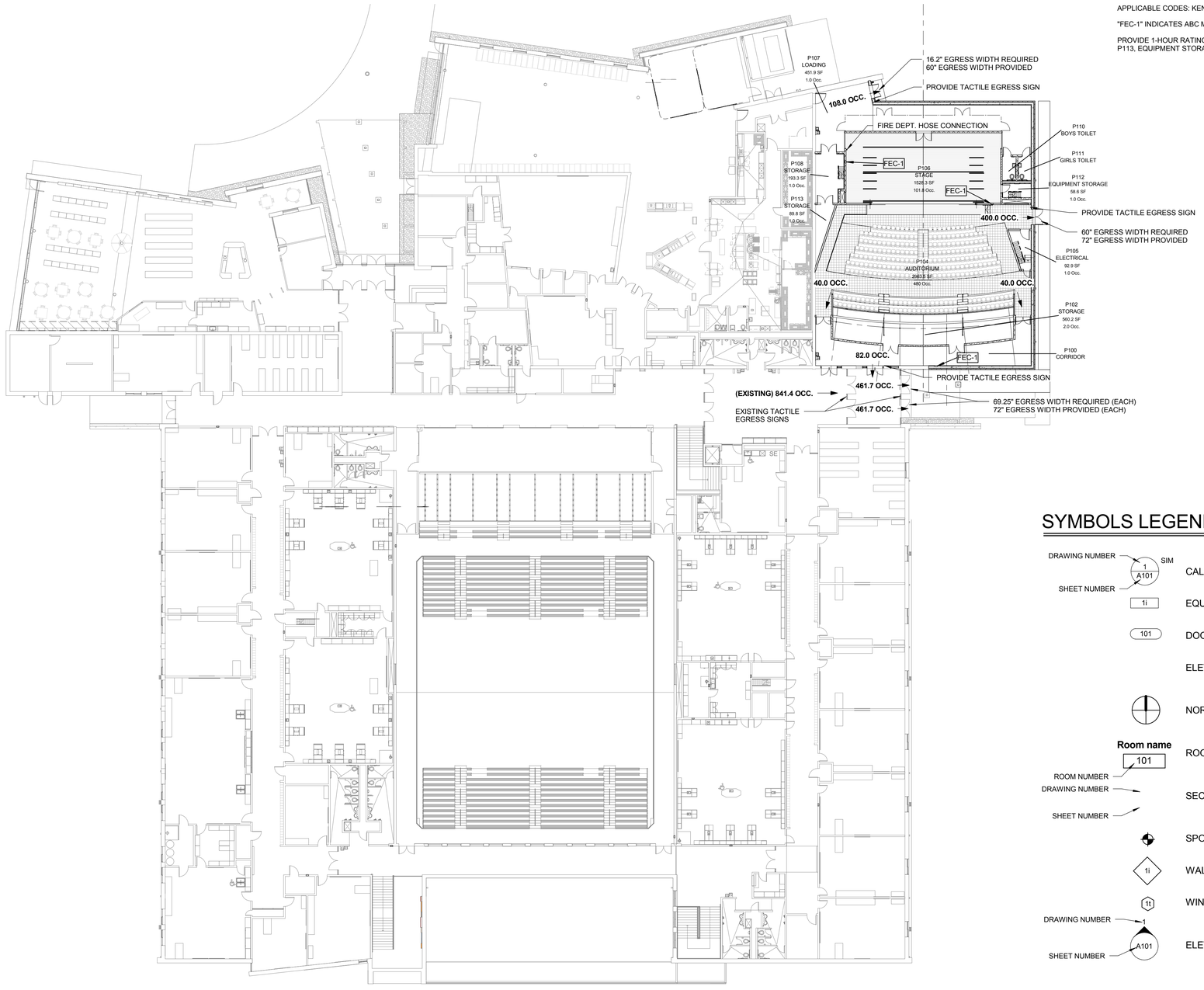

1 LEVEL 1 - LIFE SAFETY PLAN
3/64" = 1'-0"

NORTH

BUILDING CODE INFORMATION

OCCUPANCY: USE GROUP "E" (EDUCATIONAL)

CONSTRUCTION TYPE: 2B

FIRE SPRINKLER: BUILDING SHALL HAVE FULLY AUTOMATIC SPRINKLER SYSTEM

FIRE ALARM: BUILDING SHALL HAVE FULLY INTEGRATED FIRE ALARM SYSTEM

ALLOWABLE AREA: UNLIMITED

TOTAL ACTUAL AREA: 8,536 (AUDITORIUM ONLY)

OCCUPANCY LOAD: DESIGNED FOR 703 OCCUPANTS (AUDITORIUM ONLY)

APPLICABLE CODES: KENTUCKY BUILDING CODE 2006

"FEC-1" INDICATES ABC MULTI-PURPOSE FIRE EXTINGUISHER IN RECESSED CABINET

PROVIDE 1-HOUR RATING IN THE FOLLOWING ROOMS: STORAGE P108, STORAGE P113, EQUIPMENT STORAGE P112, MECH. P202 & MECH. P203).

SYMBOLS LEGEND

DRAWING NUMBER / SHEET NUMBER — 1 / A101 SIM — CALL-OUT TAG

1i — EQUIPMENT/CASEWORK/ACCESSORY TAG

101 — DOOR NUMBER TAG

ELEVATION MARKER (RELATIVE TO BUILDING)

NORTH ARROW

Room name / 101 — ROOM TAG

ROOM NUMBER / DRAWING NUMBER / SHEET NUMBER — SECTION MARKER

SPOT ELEVATION MARKER (RELATIVE TO FLOOR)

1i — WALL TAG

1i — WINDOW TAG

DRAWING NUMBER / SHEET NUMBER — 1 / A101 — ELEVATION MARKER

CONSTRUCTION DOCUMENTS

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3258 Ruckriegel Parkway, Louisville, KY 40299
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LEVEL 1 – LIFE SAFETY PLAN

NELSON COUNTY SCHOOLS - WEST CAMPUS
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GENERAL NOTES

1. PROVIDE BULL NOSE CMU UNITS AT ALL OUTSIDE CORNER EDGES.

2. ALL WALLS SHALL EXTEND AND SEAL TIGHT TO UNDERSIDE OF FLOOR/ROOF DECK U.N.O. (UNLESS NOTED OTHERWISE). PROVIDE INSULATION PACKED INTO METAL DECK FLUTES AS REQUIRED TO PROVIDE SOUND SEAL.

3. DIMENSION ORIGINS SHALL BE AS FOLLOWS:

   ICF - FACE OF INSULATION
   CMU - FACE OF CMU
   METAL STUD - FACE OF FINISH

4. REFER TO EXTERIOR ELEVATIONS, SHEETS A2.0 FOR EXTERIOR FINISHES AND ROOM FINISH SCHEDULE, SHEET A5.0 FOR INTERIOR FINISHES.

5. REFER TO STRUCTURAL DRAWINGS FOR COLUMN SIZING, WALL REINFORCEMENT AND FRAMING MEMBERS.

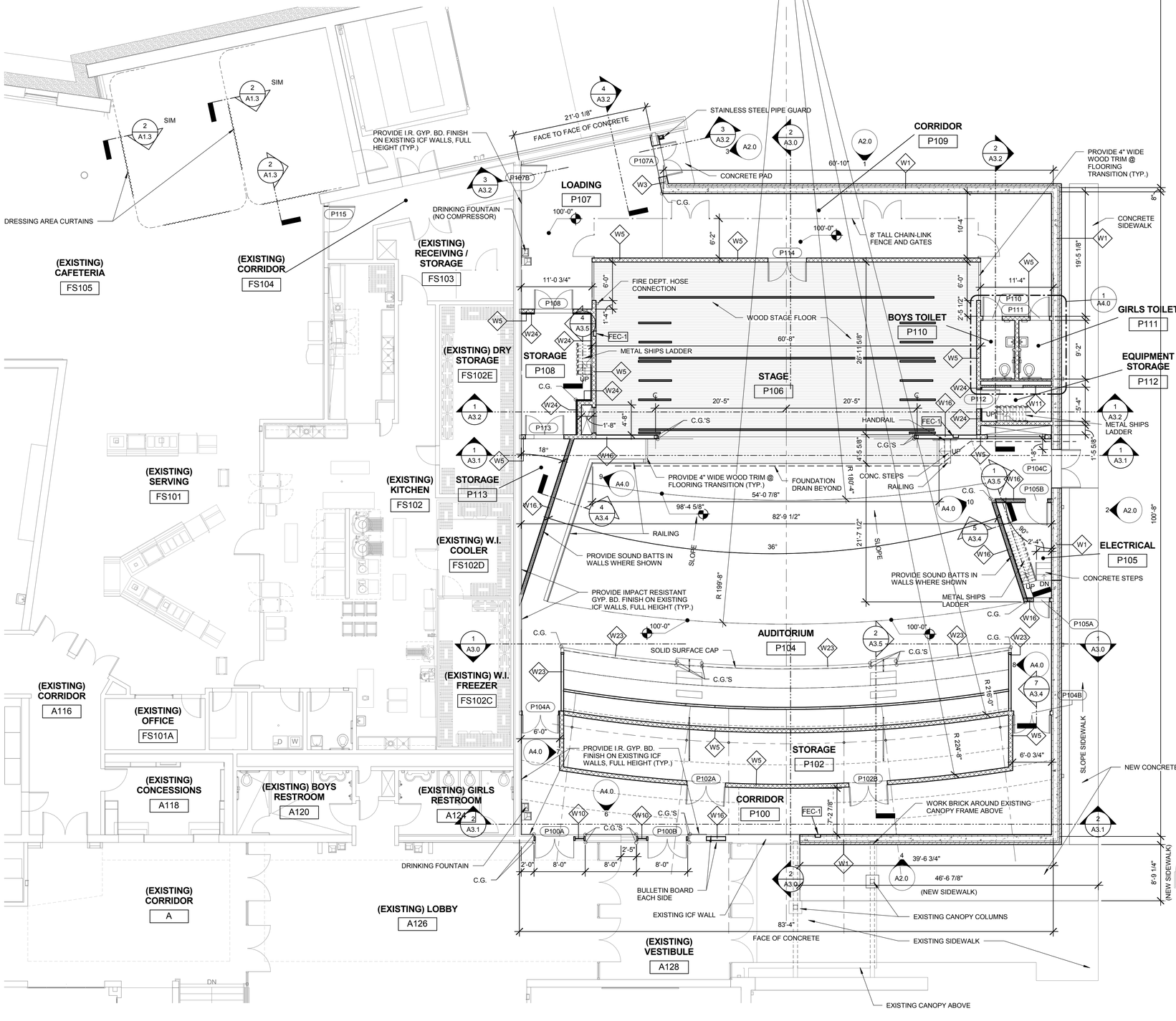
6. "CG" REFERS TO CORNER GUARDS TO BE INSTALLED

7. "SS-CG" REFERS TO STAINLESS STEEL CORNER GUARDS TO BE INSTALLED UP TO 6'-0" A.F.F.

8. REFER TO SHEET A0.1 FOR MORE INFORMATION ON FIRE EXTINGUISHERS / CABINETS.

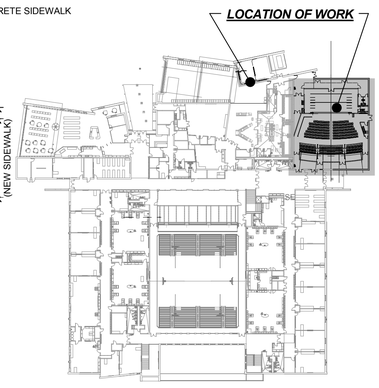
9. ALL CONCRETE SLABS ON GRADE SHALL RECEIVE A VAPOR BARRIER UNDER SLAB WITH JOINTS TAPED, PENETRATIONS SEALED AND OVERLAPS PROVIDED PER MANUFACTURERS RECOMMENDATIONS.

10. PROVIDE NEW GYPSUM BOARD FINISH ON EXISTING ICF WALLS WHERE EXTERIOR PANELS HAVE BEEN REMOVED. GYPSUM BOARD SHALL BE TO FULL HEIGHT OF WALL.



1 ENLARGED AUDITORIUM PLAN
1/8" = 1'-0"

NORTH



2 KEY PLAN
1" = 80'-0"

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NELSON COUNTY SCHOOL DISTRICT
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FLOOR PLAN
NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

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WALL SCHEDULE		
Type Mark		Description
W1		ICF 8" - 5/8" IMPACT RESISTANT GYP. BD. INTERIOR - BRICK VENEER OVER BUILDING WRAP EXTERIOR
W3		ICF 8" - 5/8" IMPACT RESISTANT GYP. BD. INTERIOR - METAL PANEL OVER BUILDING WRAP EXTERIOR
W4		ICF 8" - 5/8" IMPACT RESISTANT GYP. BD. EACH SIDE
W5		8" CMU - EXTEND UP TO UNDERSIDE OF DECK
W10		2" X 4 1/2" NON-THERMAL ALUMINUM STOREFRONT SYSTEM - 1/4" CLEAR GLAZING
W11		4" CMU - EXTEND UP TO UNDERSIDE OF DECK
W15		3 5/8" STEEL STUDS @ 16" O.C. - 5/8" GYP. BD. ONE SIDE. EXTEND UP TO UNDERSIDE OF DECK
W16		6" STEEL STUDS @ 16" O.C. - 5/8" IMPACT RESISTANT GYP. BD. EACH SIDE. EXTEND UP TO UNDERSIDE OF DECK
W16.1		1-HOUR RATED, 6" STEEL STUDS @ 16" O.C. - 5/8" IMPACT RESISTANT TYPE "X" GYP. BD. EACH SIDE. EXTEND UP TO UNDERSIDE OF DECK (UL DES 419)
W23		3 5/8" STEEL STUDS @ 16" O.C. - 5/8" GYP. BD. EACH SIDE
W24		NOISE ISOLATION WALL - (2) LAYERS TYPE "X" GYP. BD. OVER ISOLATION CLIPS WITH 1" RIGID INSULATION BETWEEN CLIPS - SEE SECTION 4/A3.3 FOR MORE INFORMATION

GENERAL NOTES

1. PROVIDE BULL NOSE CMU UNITS AT ALL OUTSIDE CORNER EDGES.

2. ALL WALLS SHALL EXTEND AND SEAL TIGHT TO UNDERSIDE OF FLOOR/ROOF DECK U.N.O. (UNLESS NOTED OTHERWISE). PROVIDE INSULATION PACKED INTO METAL DECK FLUTES AS REQUIRED TO PROVIDE SOUND SEAL.

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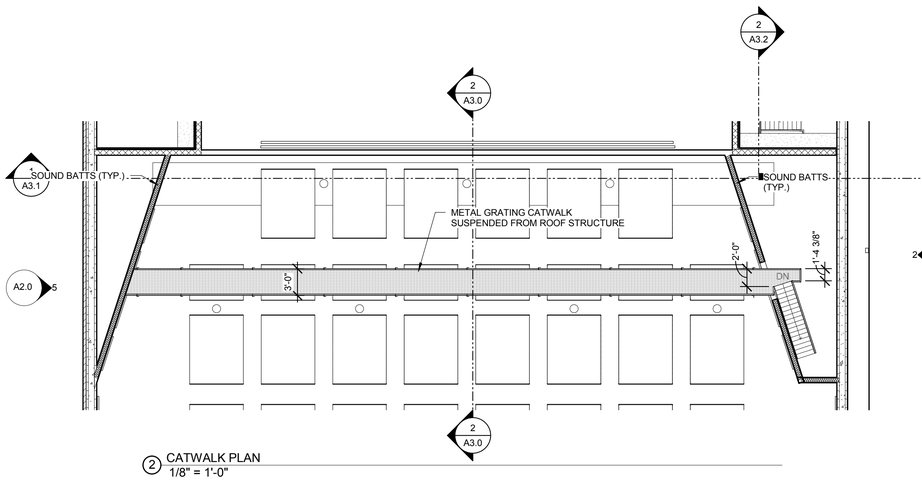
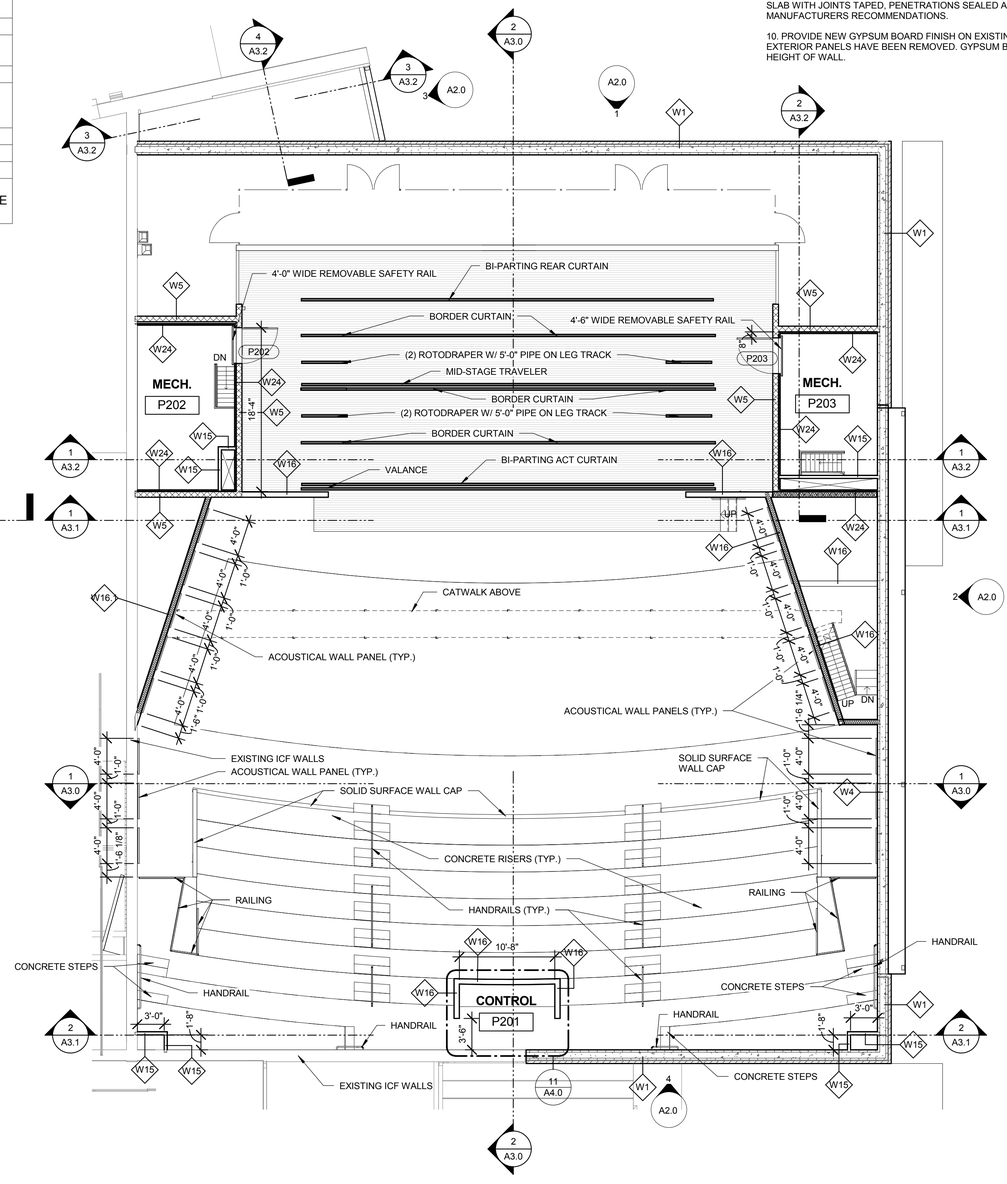
10. PROVIDE NEW GYPSUM BOARD FINISH ON EXISTING ICF WALLS WHERE EXTERIOR PANELS HAVE BEEN REMOVED. GYPSUM BOARD SHALL BE TO FULL HEIGHT OF WALL.



② CATWALK PLAN
1/8" = 1'-0"



① AUDITORIUM MEZZANINE
1/8" = 1'-0"

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MEZZANINE LEVEL FLOOR PLAN

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

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CONSTRUCTION DOCUMENTS

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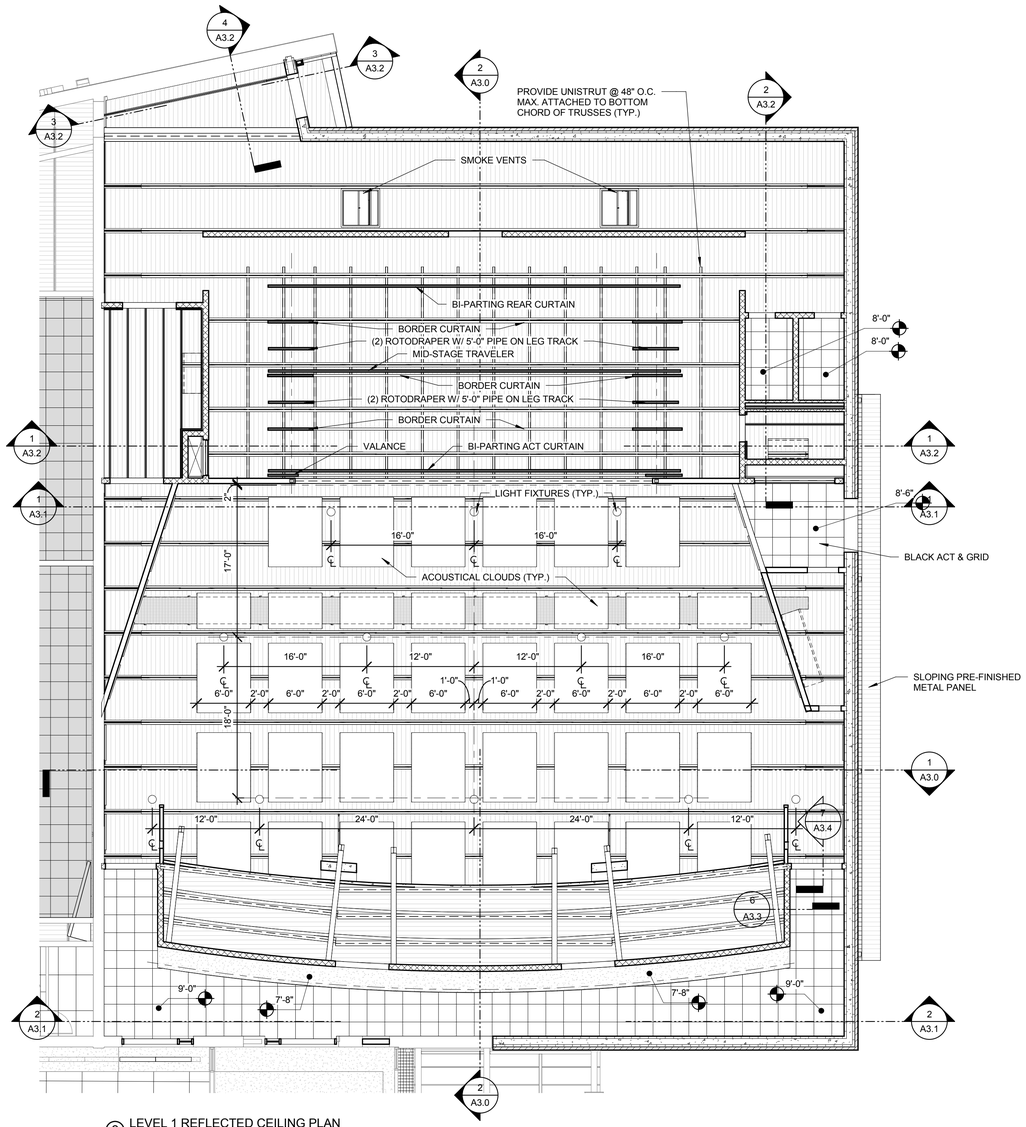
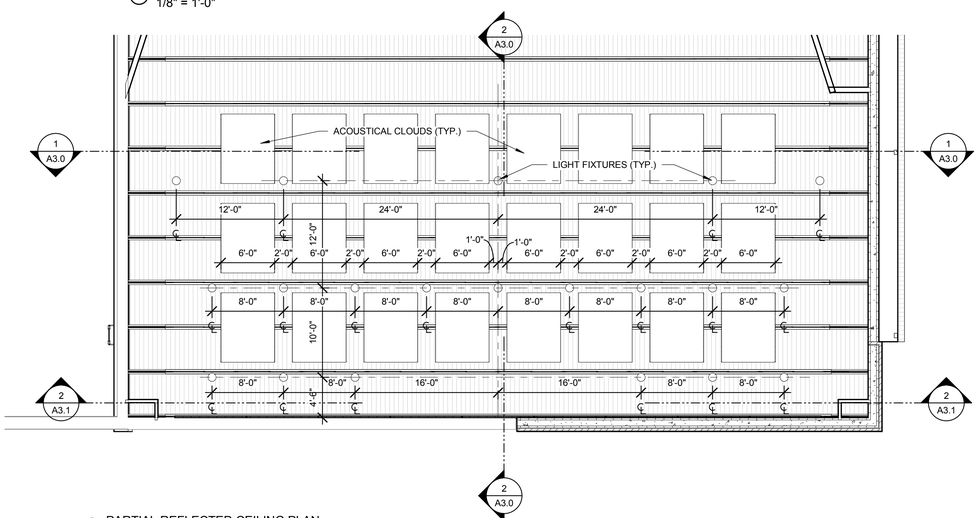
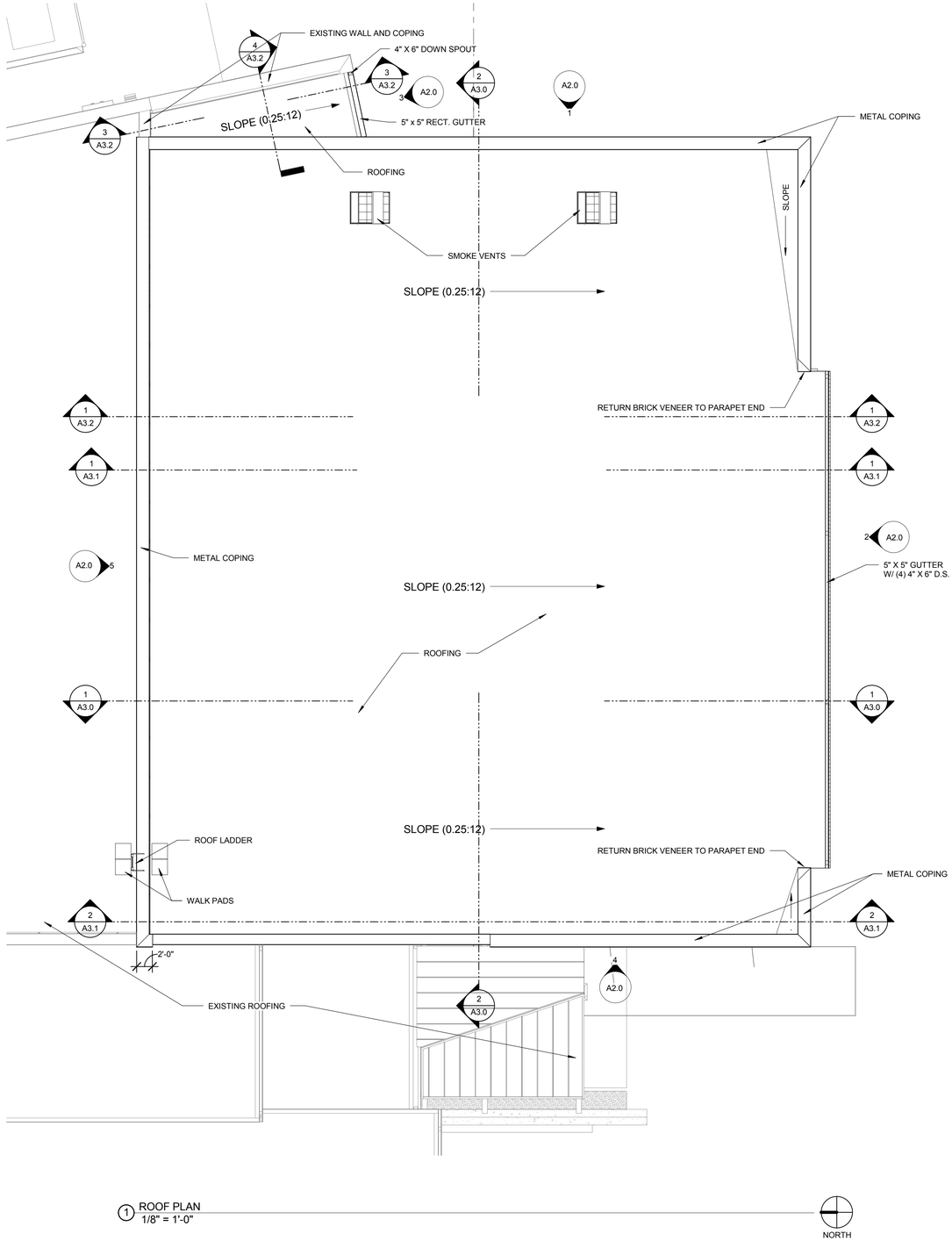


2 LEVEL 1 REFLECTED CEILING PLAN
1/8" = 1'-0"



3 PARTIAL REFLECTED CEILING PLAN
1/8" = 1'-0"



1 ROOF PLAN
1/8" = 1'-0"

CONSTRUCTION DOCUMENTS

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NELSON COUNTY SCHOOL DISTRICT
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ROOF AND CEILING PLANS
NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

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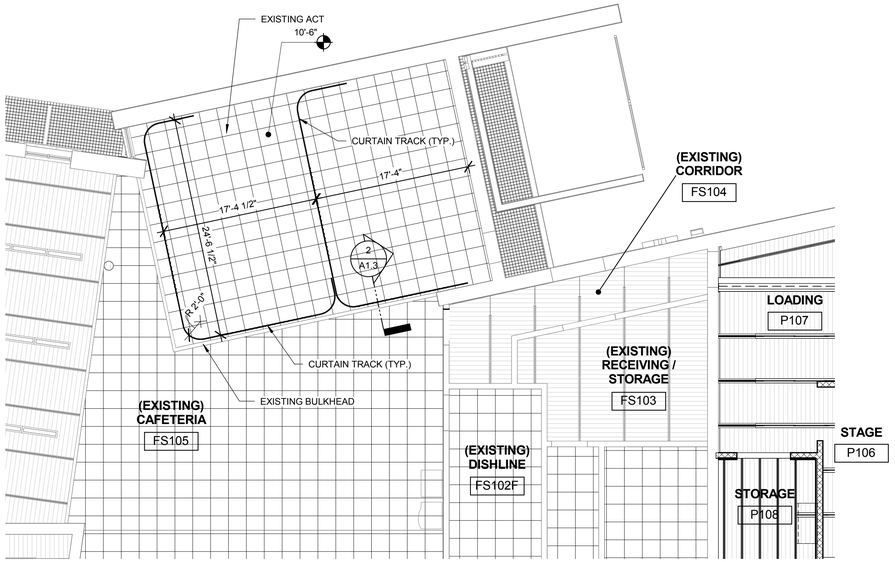
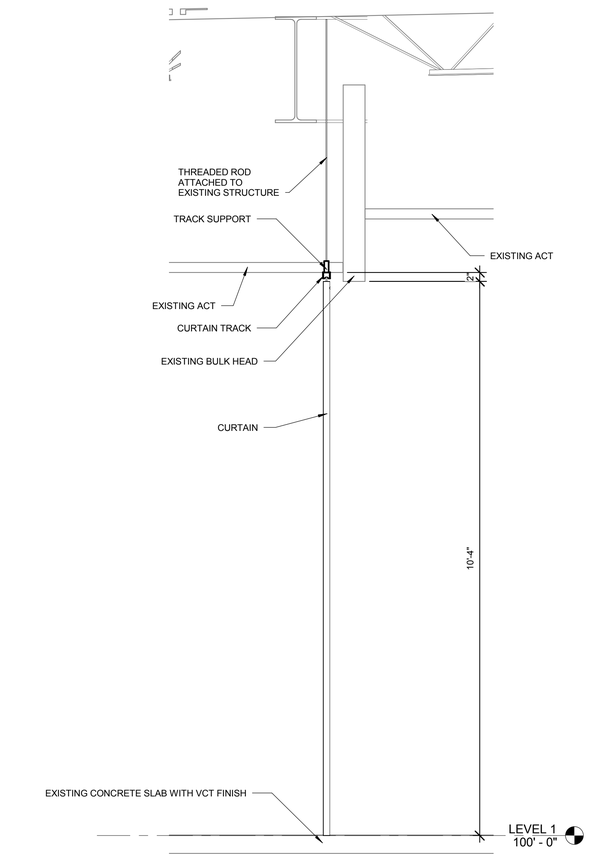


1 PARTIAL REFLECTED CEILING PLAN
1/8" = 1'-0"



2 CURTAIN SECTION
3/4" = 1'-0"

CONSTRUCTION DOCUMENTS

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NELSON COUNTY SCHOOL DISTRICT
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DRESSING AREA CEILING PLAN & DETAILS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
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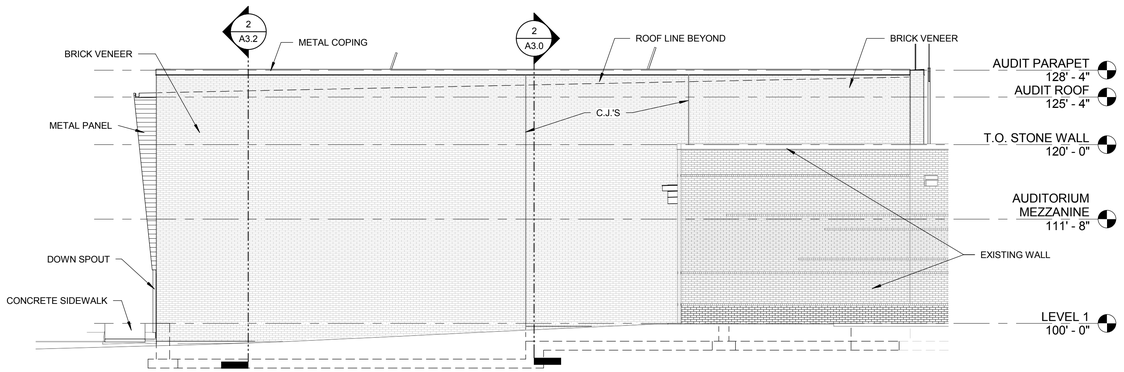
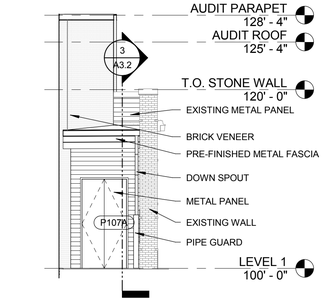
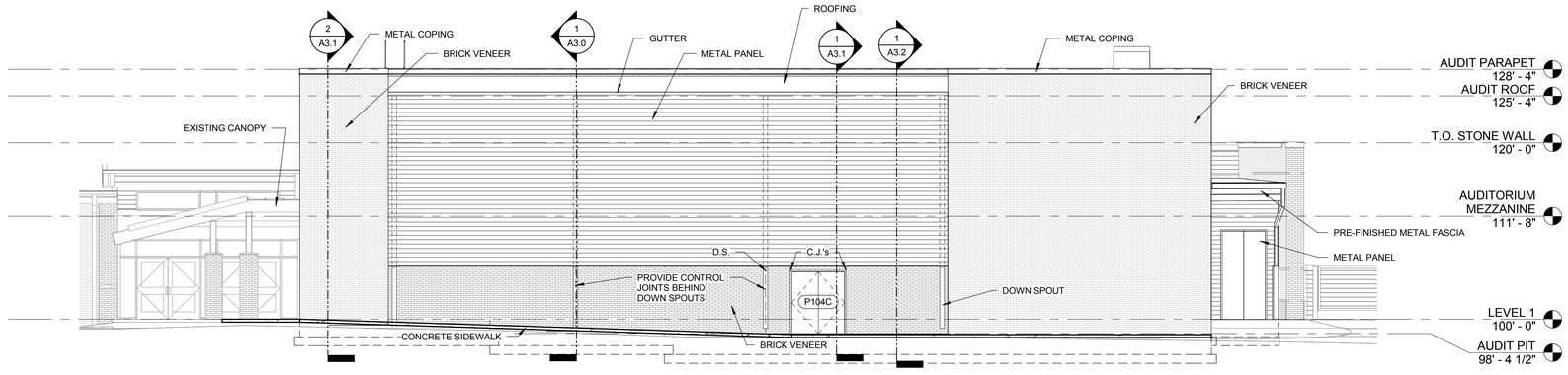


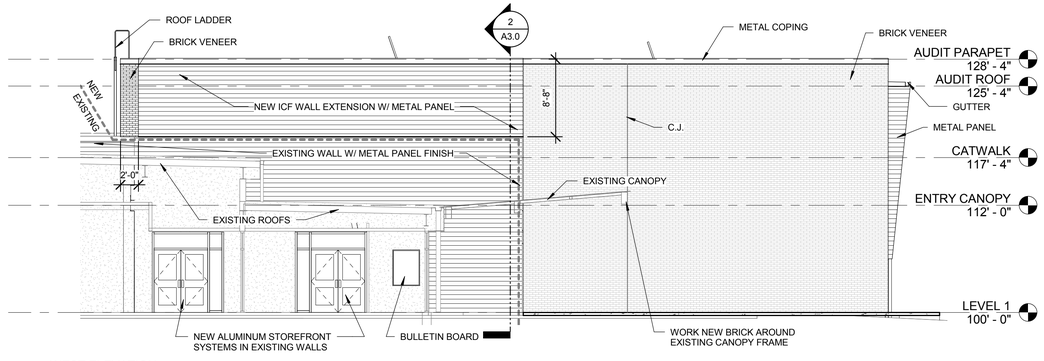
1 EAST ELEVATION
1/8" = 1'-0"



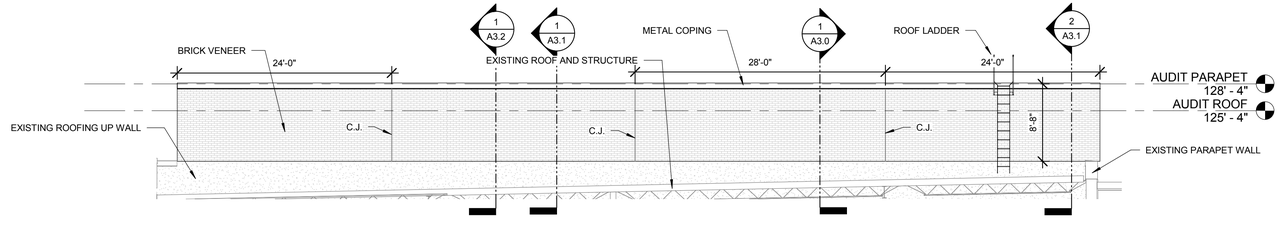
3 EXTERIOR ELEVATION
1/8" = 1'-0"



2 SOUTH ELEVATION
1/8" = 1'-0"



4 WEST ELEVATION
1/8" = 1'-0"



5 EXTERIOR ELEVATION
1/8" = 1'-0"

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NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
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CONSTRUCTION DOCUMENTS

EXTERIOR ELEVATIONS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
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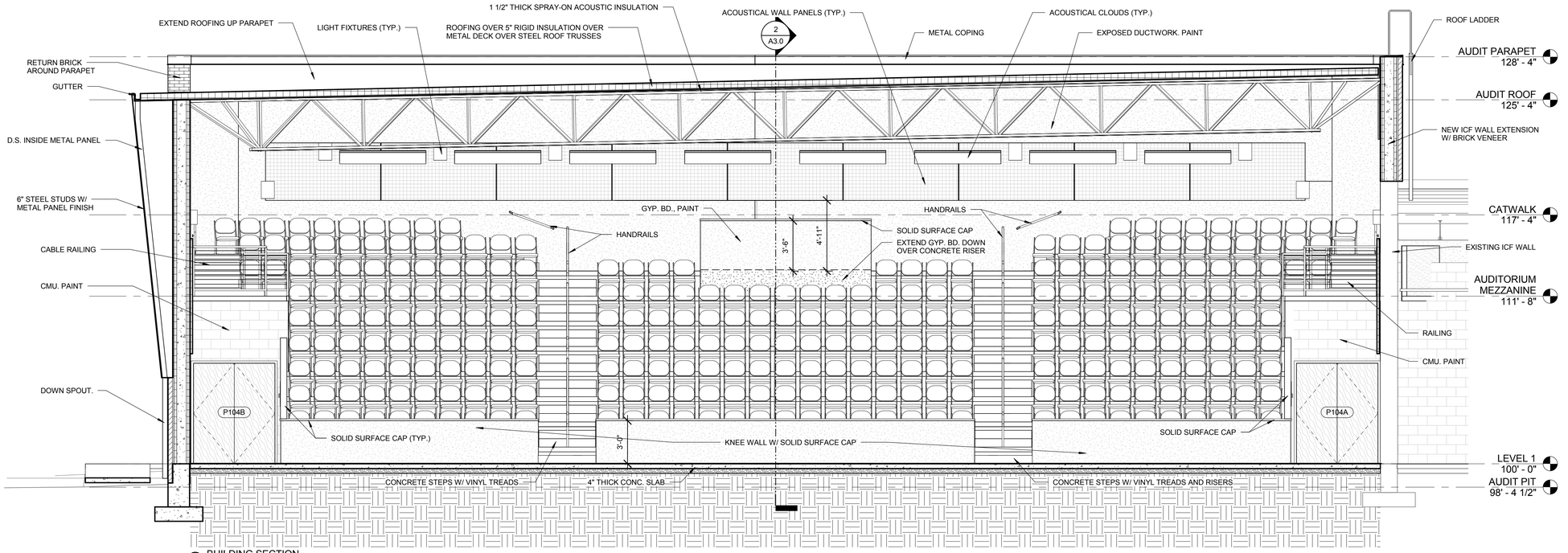
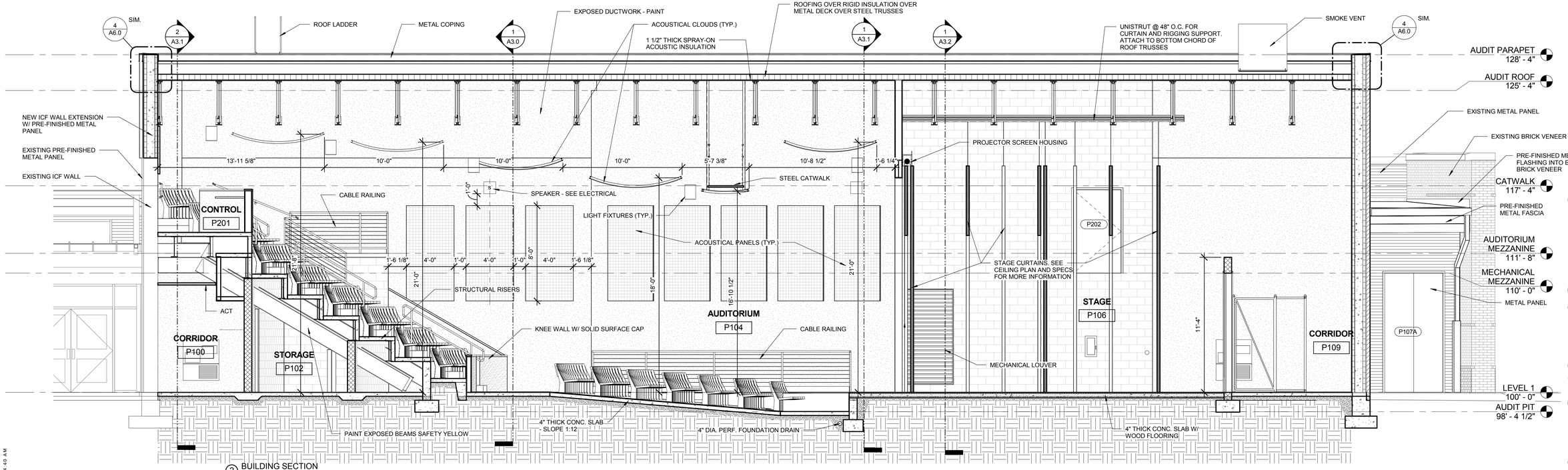


1 BUILDING SECTION
1/4" = 1'-0"



2 BUILDING SECTION
1/4" = 1'-0"

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NELSON COUNTY SCHOOL DISTRICT
288 WILDCAT LANE
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BUILDING SECTIONS

NELSON COUNTY SCHOOLS - WEST CAMPUS
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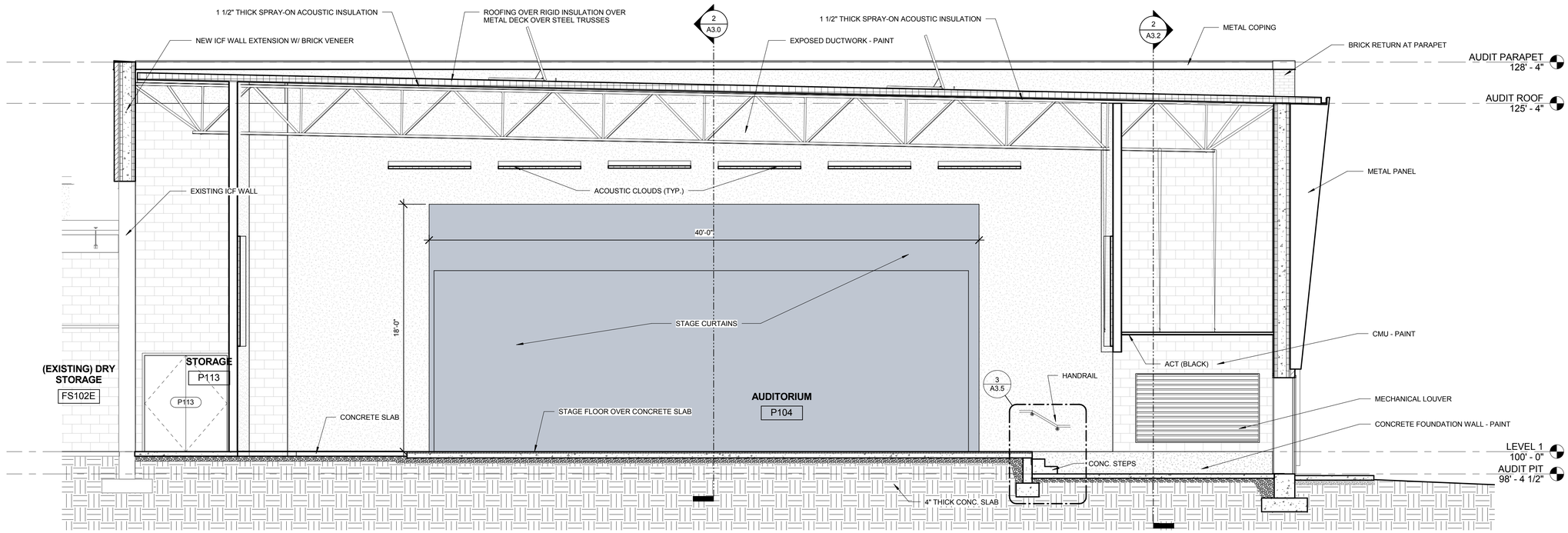
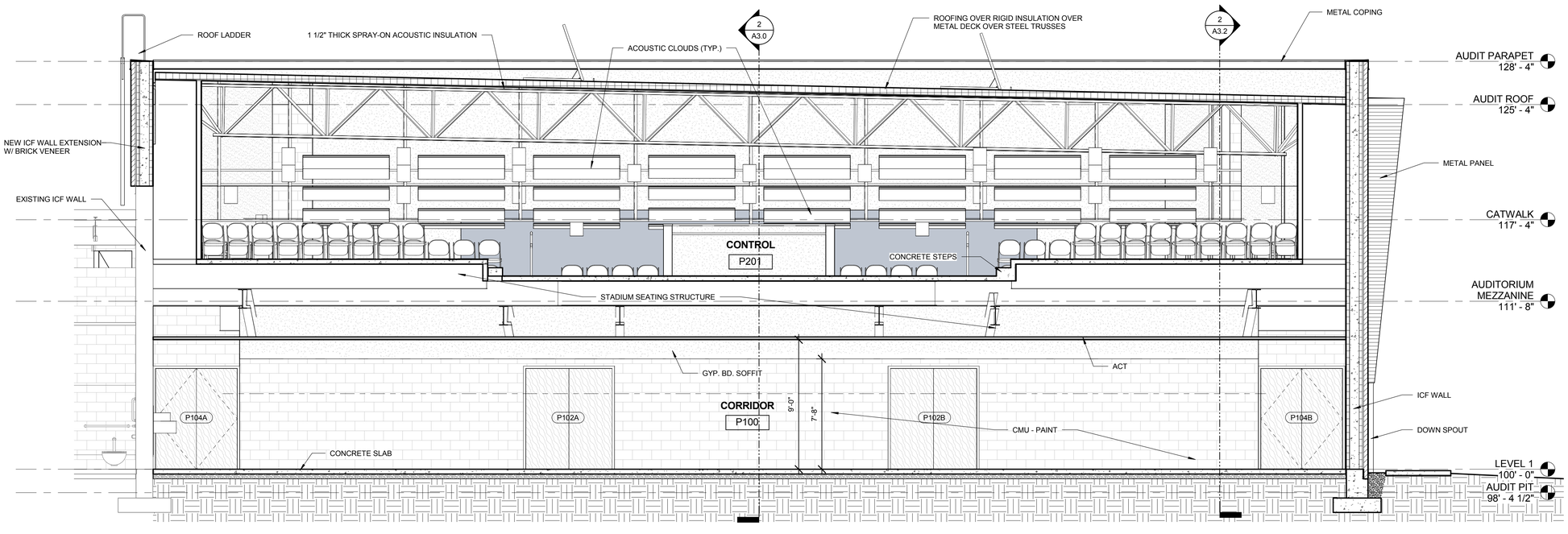


1 BUILDING SECTION
1/4" = 1'-0"



2 BUILDING SECTION
1/4" = 1'-0"

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CONSTRUCTION DOCUMENTS

BUILDING SECTIONS
NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

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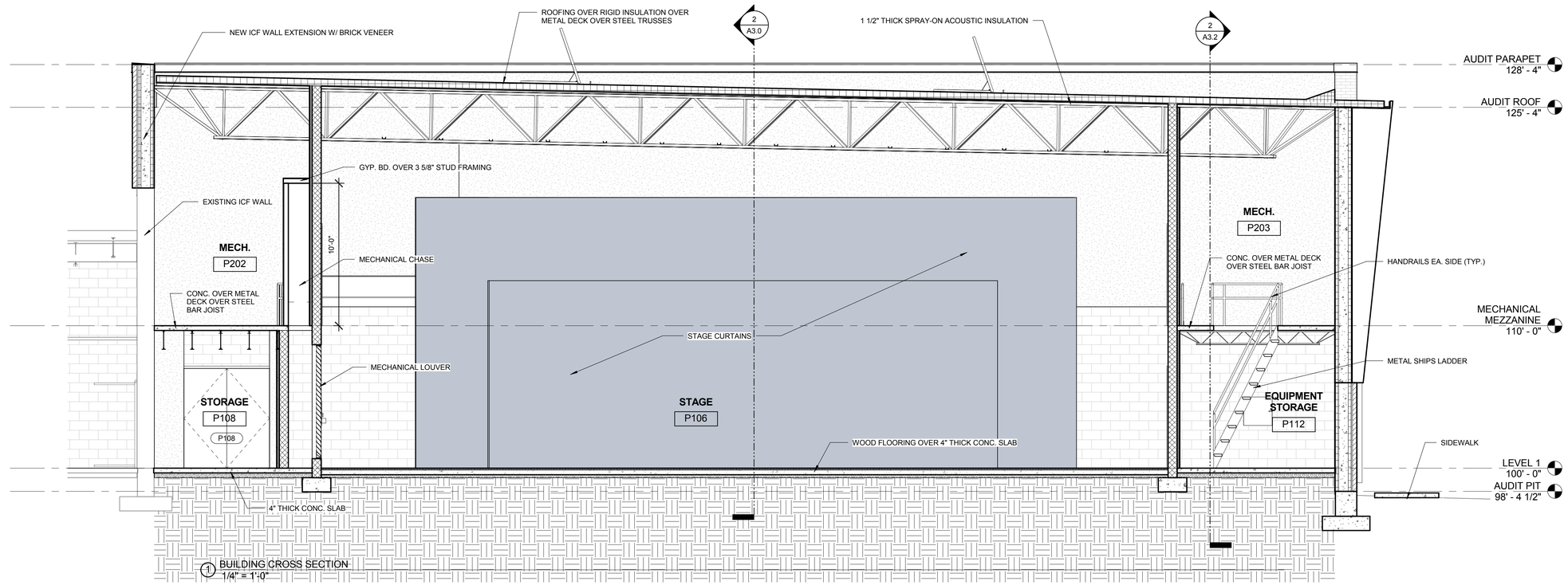
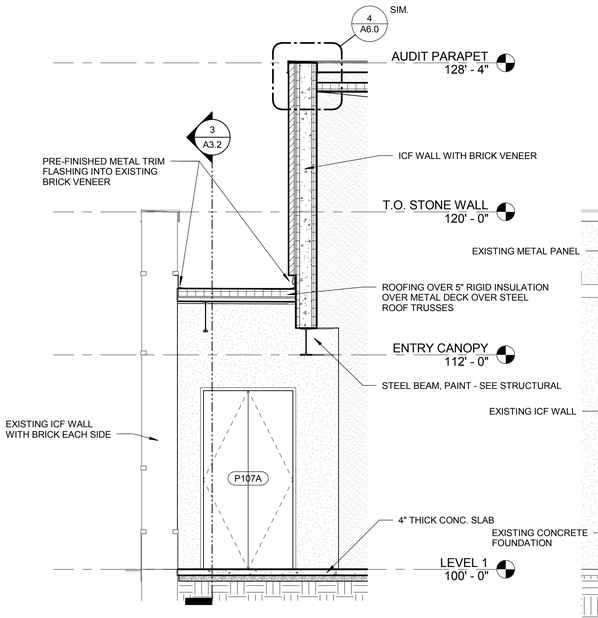
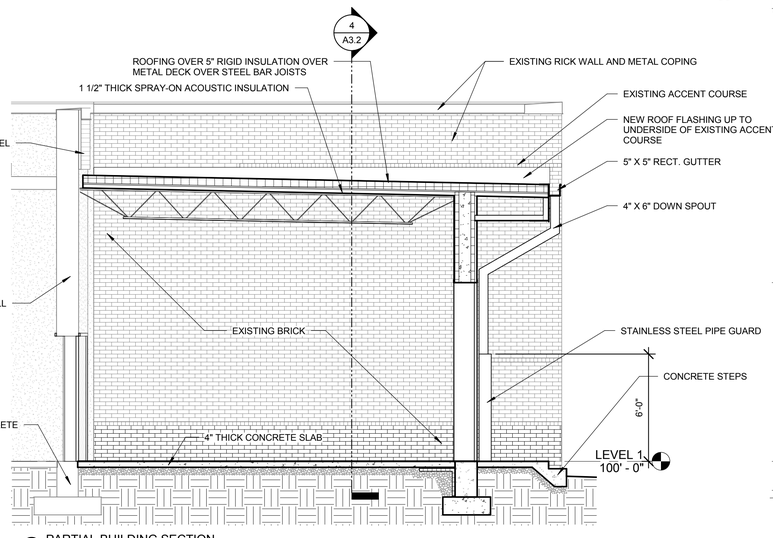


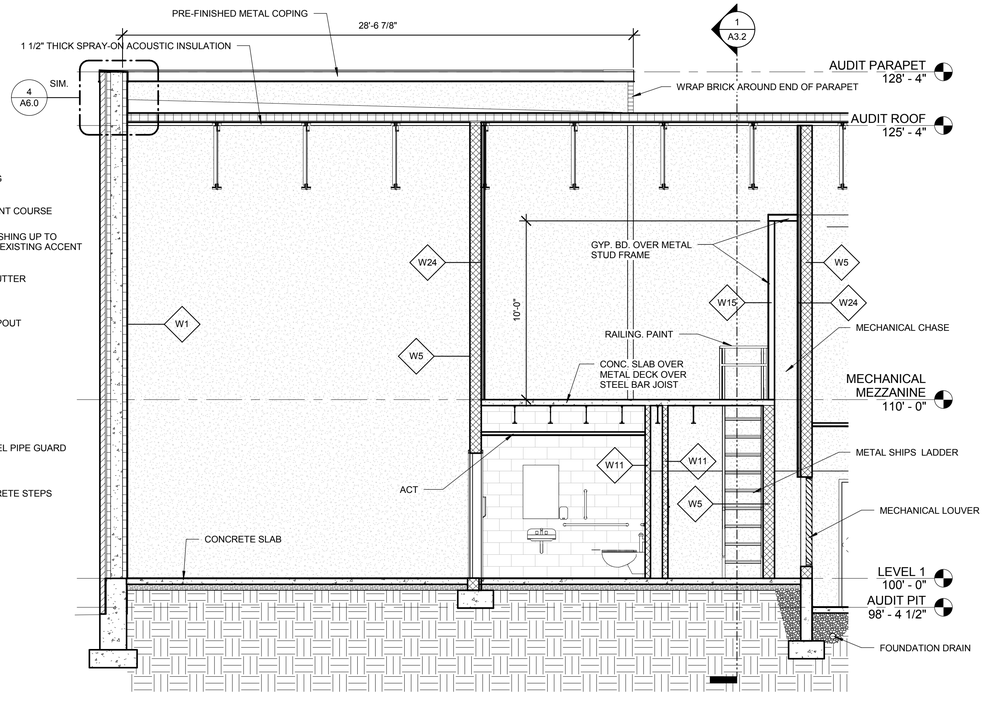
1 BUILDING CROSS SECTION
1/4" = 1'-0"



4 BUILDING SECTION
1/4" = 1'-0"



3 PARTIAL BUILDING SECTION
1/4" = 1'-0"



2 PARTIAL BUILDING SECTION
1/4" = 1'-0"

CONSTRUCTION DOCUMENTS

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BUILDING SECTIONS
NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

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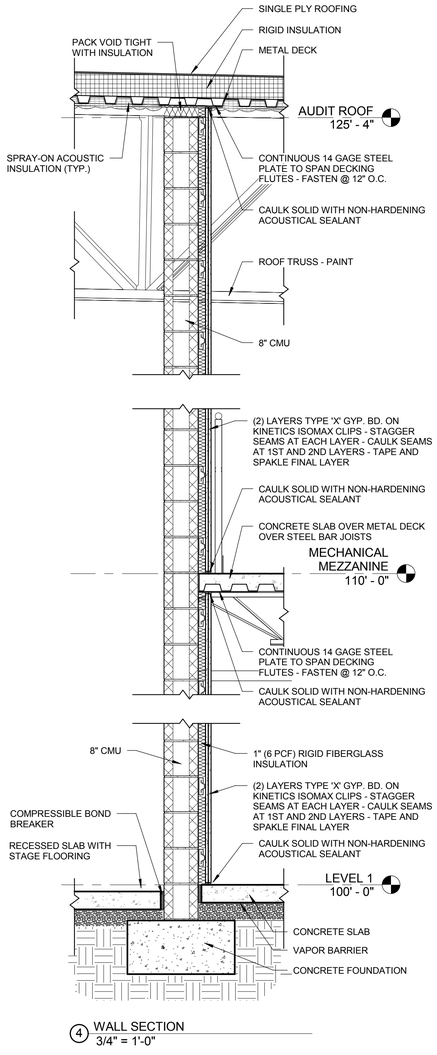
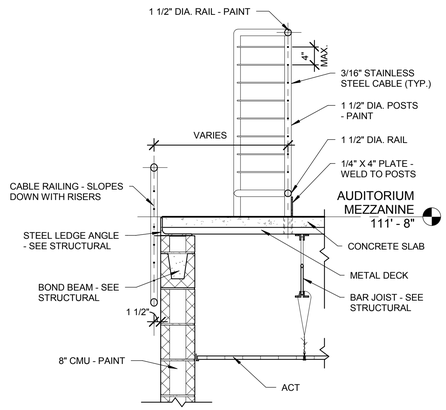
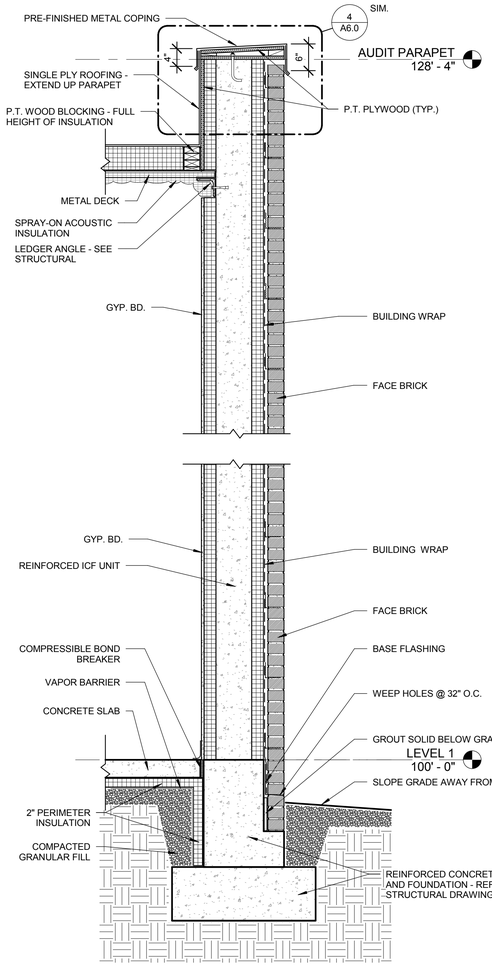


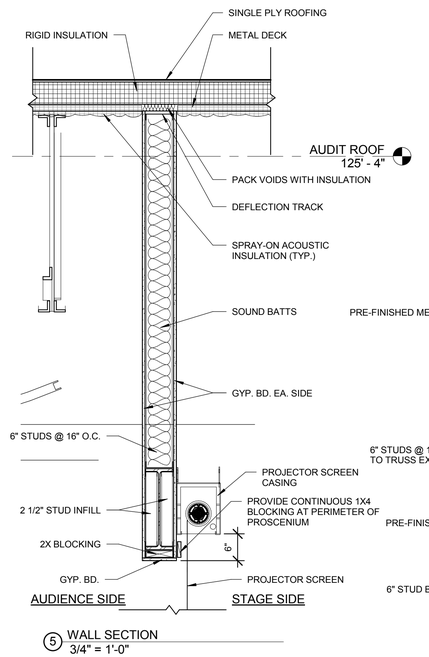
4 WALL SECTION
3/4" = 1'-0"



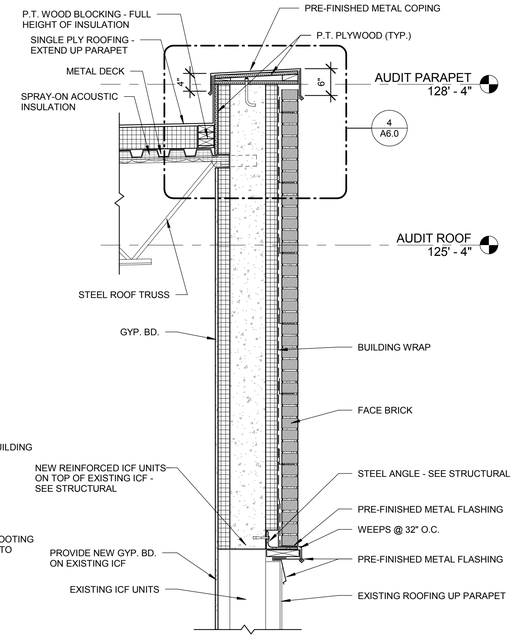
6 FLOOR/RAILING DETAIL
3/4" = 1'-0"



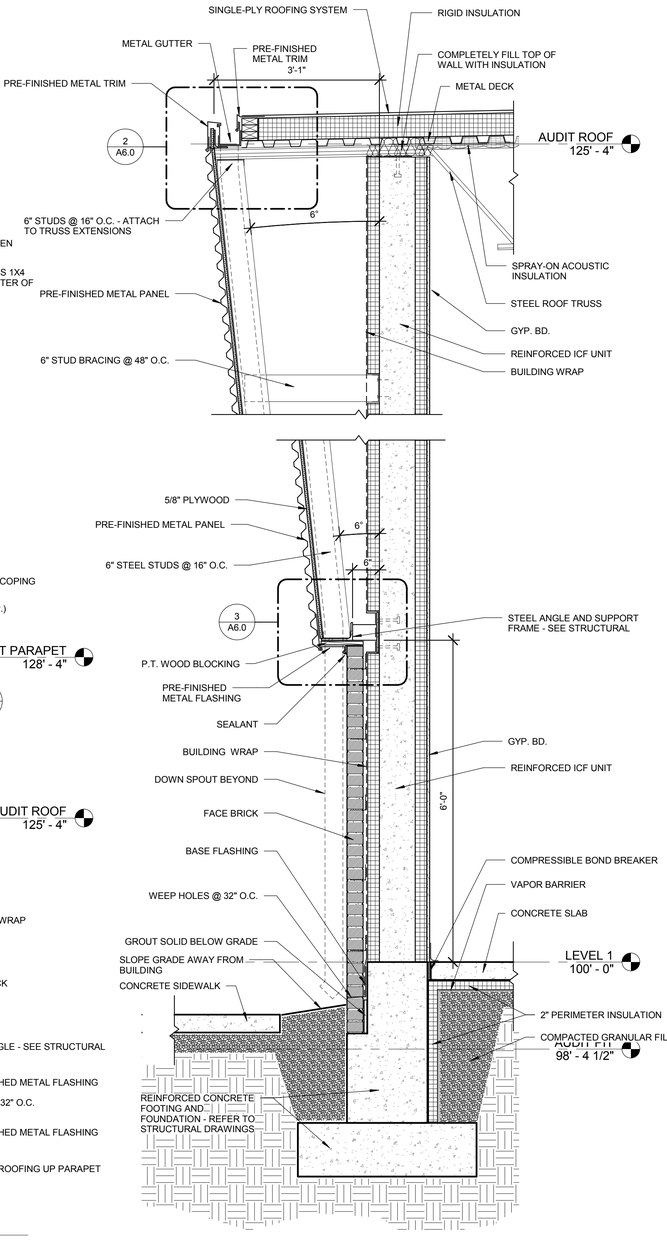
3 WALL SECTION
3/4" = 1'-0"



5 WALL SECTION
3/4" = 1'-0"



2 WALL SECTION
3/4" = 1'-0"



1 WALL SECTION
3/4" = 1'-0"

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WALL SECTIONS

NELSON COUNTY SCHOOLS - WEST CAMPUS
THOMAS NELSON H.S.
PHASE III
2885 NEW SHEPHERDSVILLE ROAD (HWY 245) BARDSTOWN, KENTUCKY 40004

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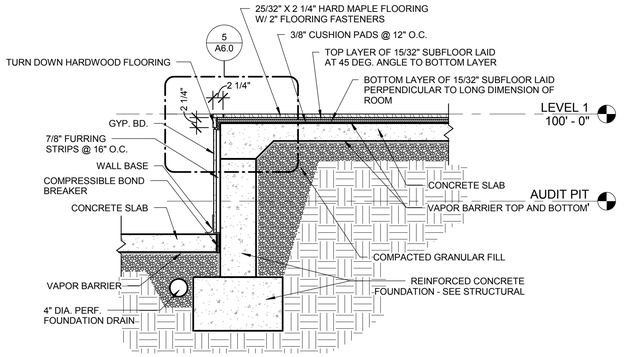
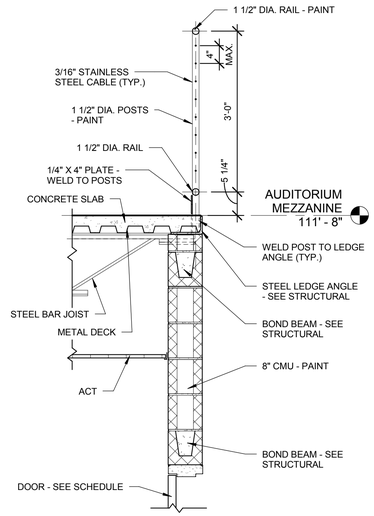
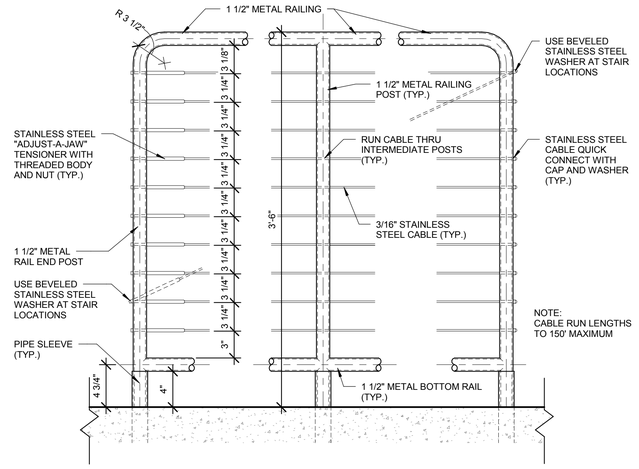


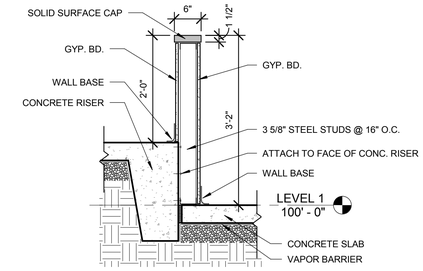
8 WALL SECTION
3/4" = 1'-0"



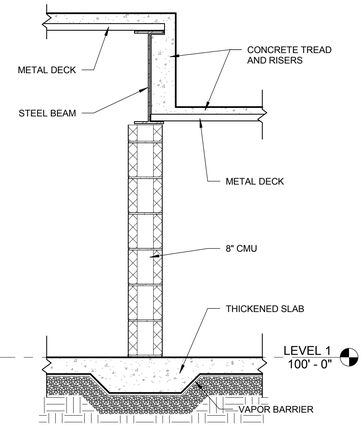
7 WALL SECTION
3/4" = 1'-0"



6 CABLE RAILING ELEVATION / DETAIL
1 1/2" = 1'-0"

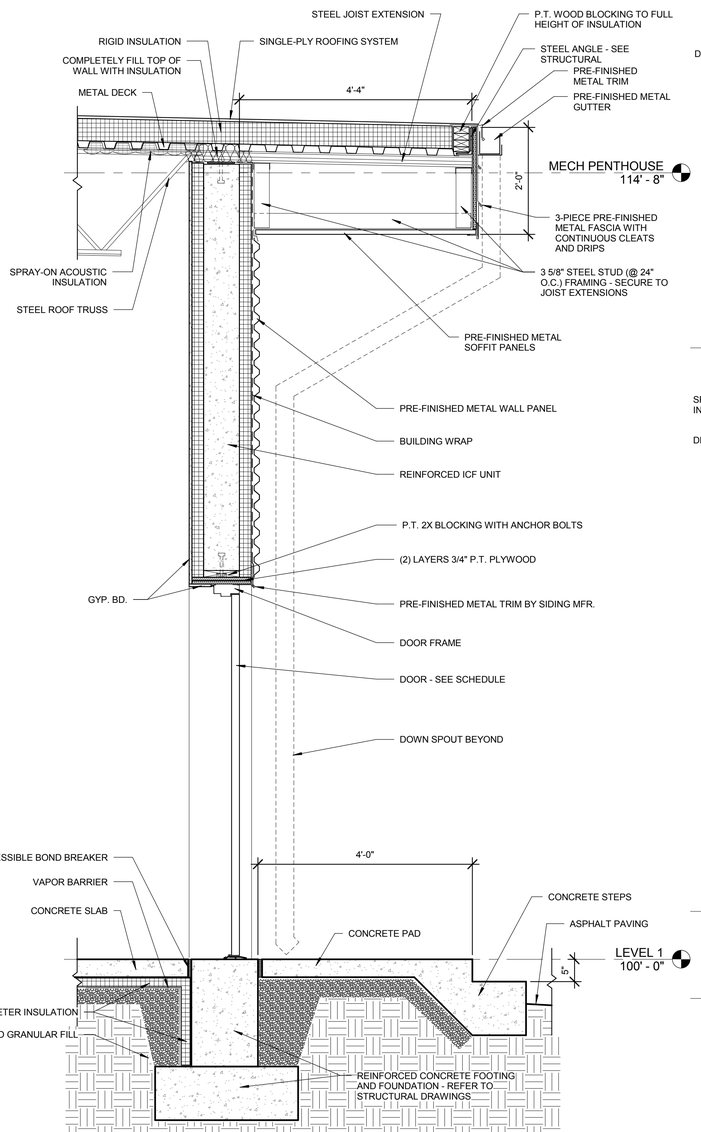


1 WALL SECTION
3/4" = 1'-0"

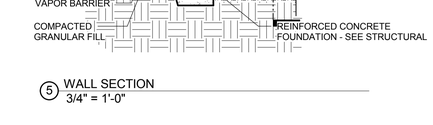


2 WALL SECTION
3/4" = 1'-0"

3 WALL SECTION
3/4" = 1'-0"



9 A3.4/9 - LOADING
3/4" = 1'-0"



5 WALL SECTION
3/4" = 1'-0"

4 WALL SECTION
3/4" = 1'-0"

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WALL SECTIONS
NELSON COUNTY SCHOOLS - WEST CAMPUS
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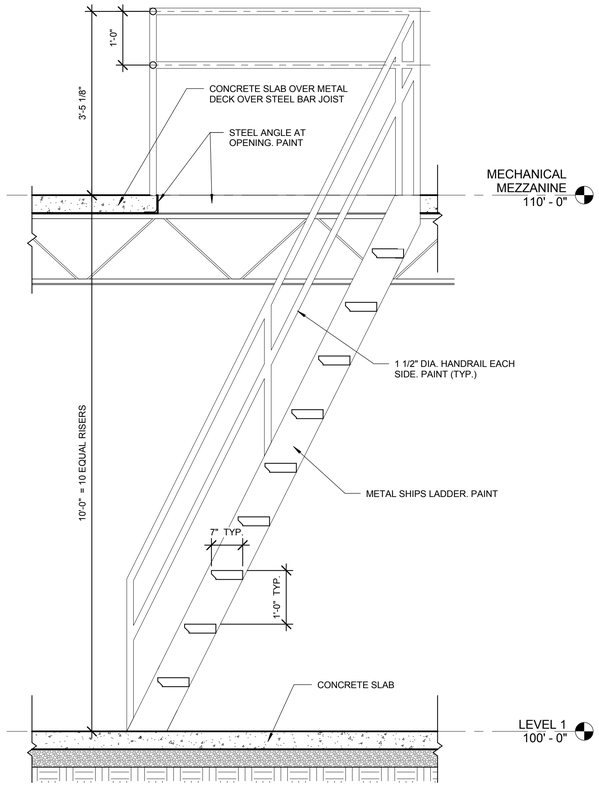
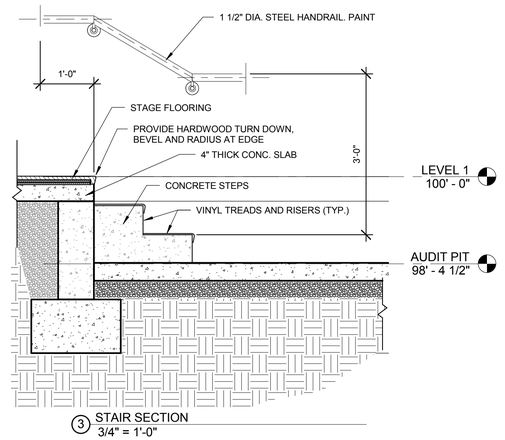
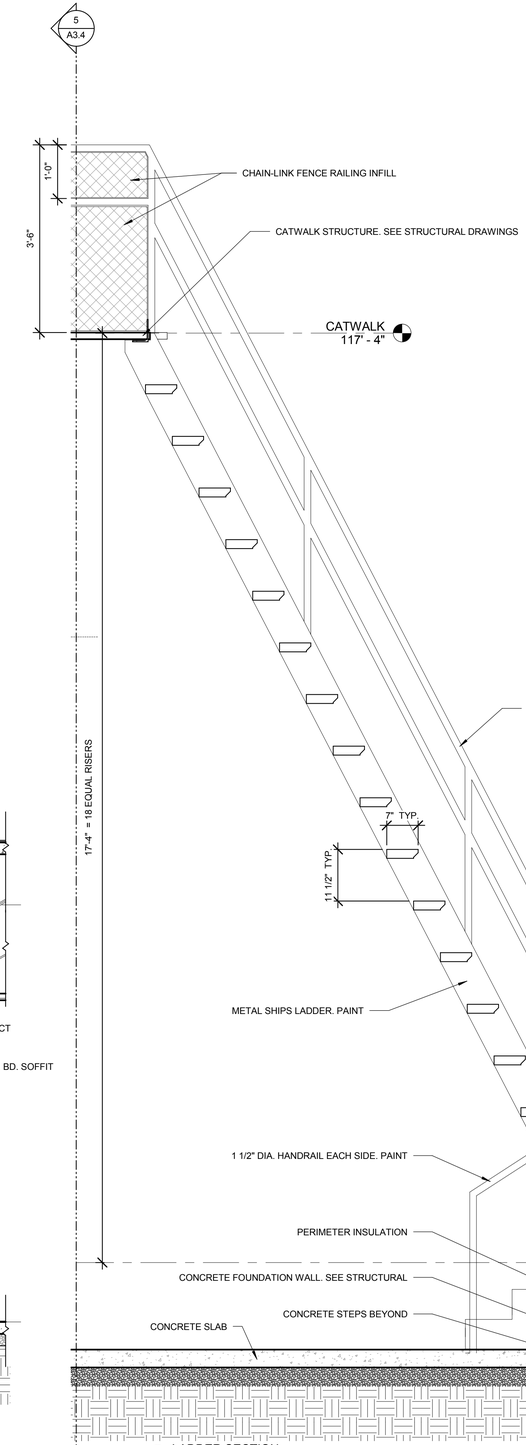


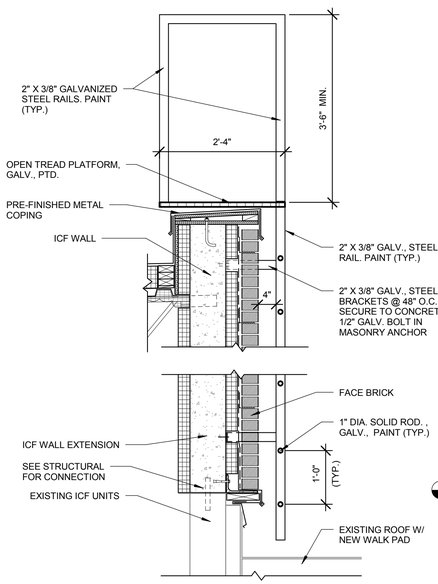
4 LADDER SECTION
3/4" = 1'-0"



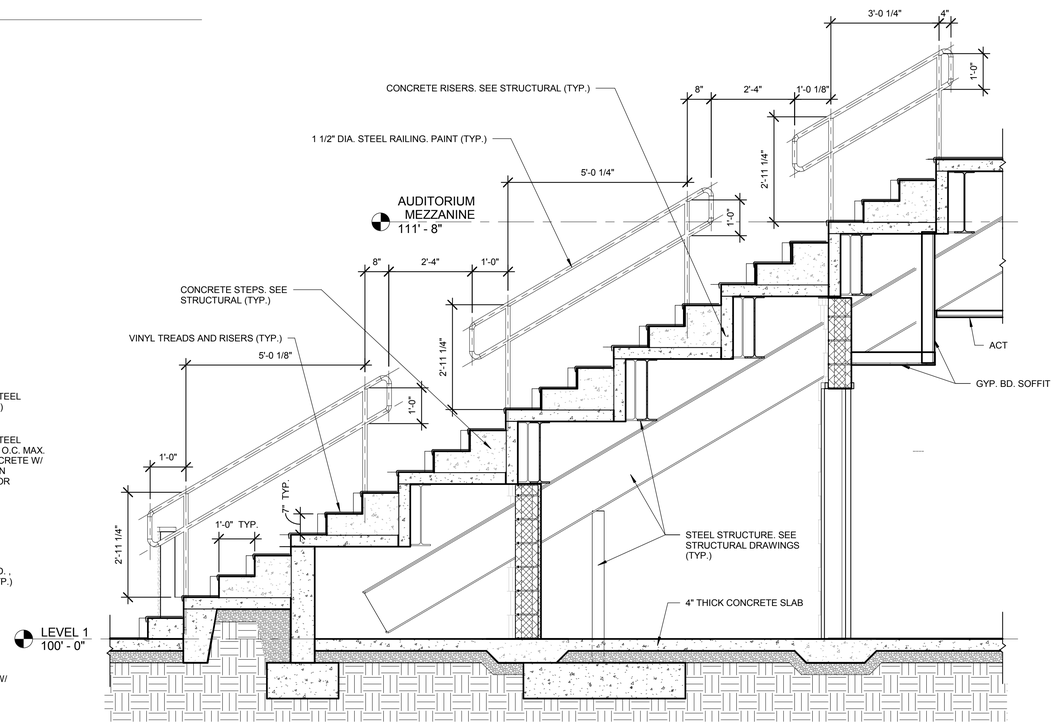
3 STAIR SECTION
3/4" = 1'-0"



1 LADDER SECTION
3/4" = 1'-0"



5 TYPICAL ROOF LADDER DETAIL
3/4" = 1'-0"



2 STEPS SECTION
1/2" = 1'-0"

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TEL 502.499.1100 FAX 499.1101

EDWARD KREMER SCOTT — Registered Architect — Commonwealth of Kentucky

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CONSTRUCTION DOCUMENTS

STAIR/LADDER SECTIONS

NELSON COUNTY SCHOOLS - WEST CAMPUS
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